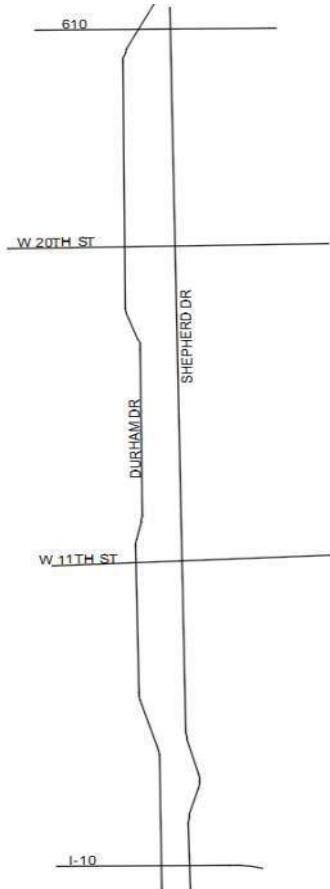


Shepherd Drive and Durham Drive

Traffic Analysis

December 2017



Prepared for:
Memorial Heights Redevelopment Authority

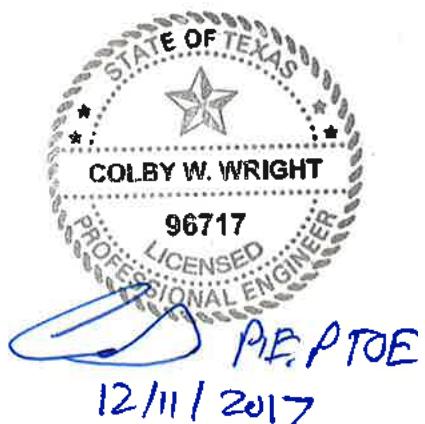
Prepared by:



Texas Board of Professional Engineers Registration No. F-439
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Shepherd Drive and Durham Drive

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JONES | CARTER

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Existing Conditions

A site visit was conducted to document the existing conditions of the study area roadways and site. **Figure 1** shows existing lane assignments and traffic control at the study area intersections.

Roadways

Shepherd Drive is a one-way northbound roadway that is classified as a major thoroughfare. It is a concrete roadway and 46' wide with four lanes in the northbound direction, and a posted speed limit of 35 mph.

Durham Drive is a one-way southbound roadway that is classified as a major thoroughfare. It is a concrete roadway and 44' wide with four lanes in the southbound direction, and a posted speed limit of 35 mph.

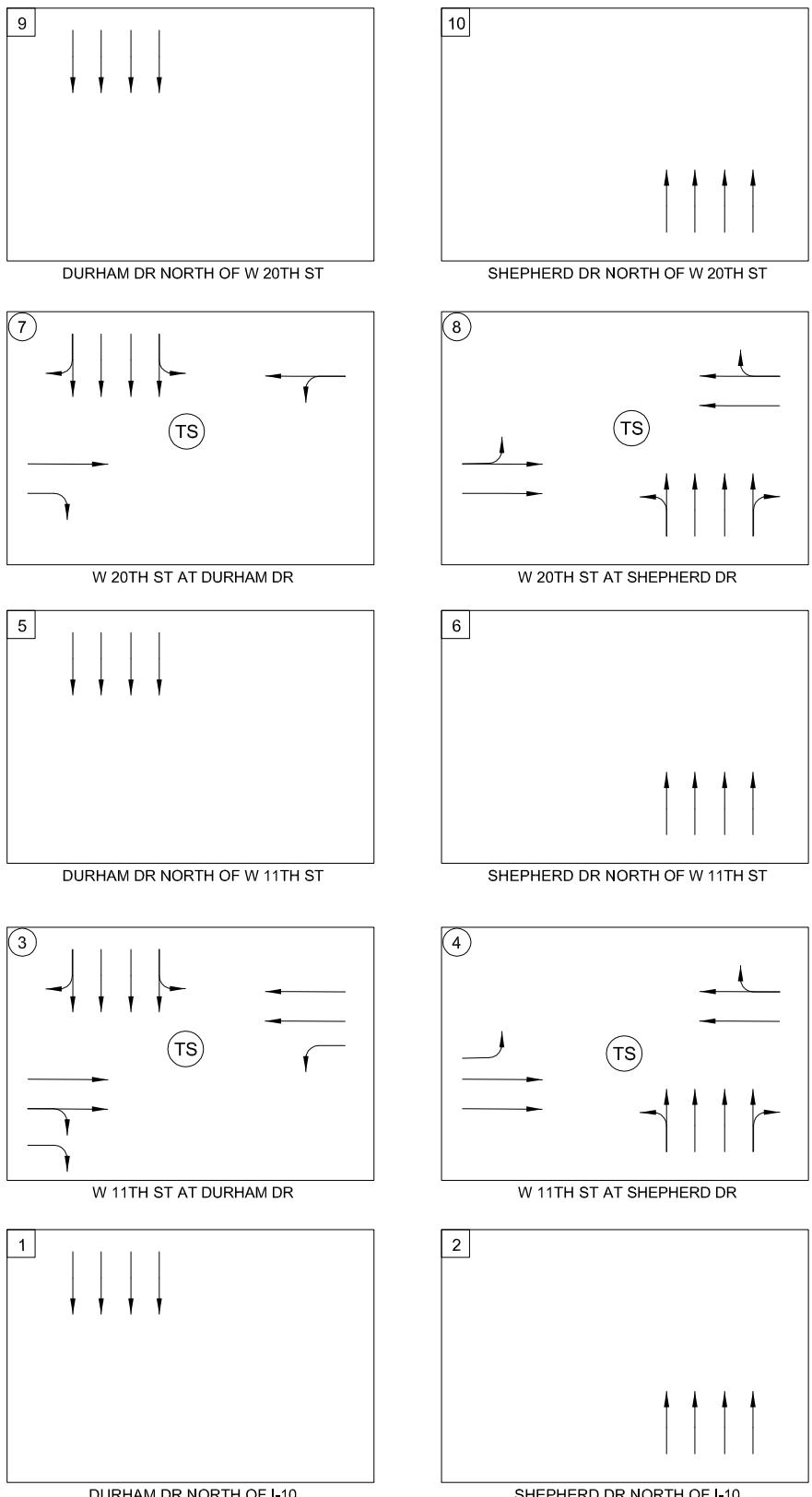
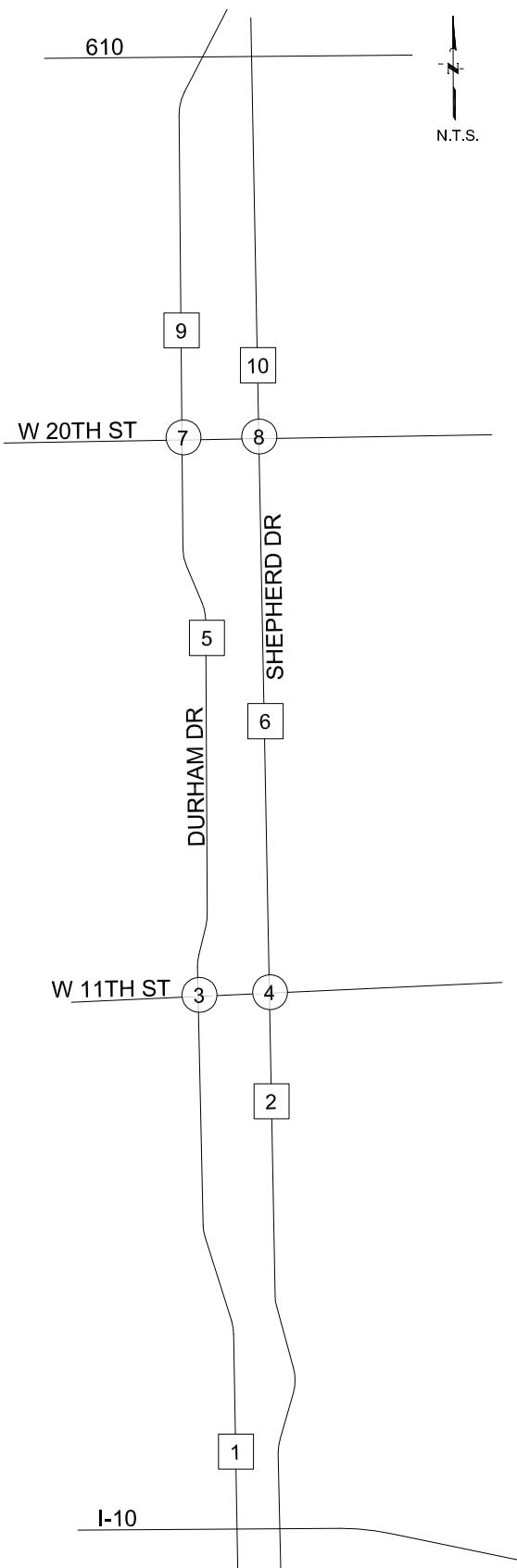
Intersections

The intersections included in the study area are:

- W 11th Street at Shepherd Drive
- W 11th Street at Durham Drive
- W 20th Street at Shepherd Drive
- W 20th Street at Durham Drive

Traffic Data

Peak hour turning movement counts and 24-hour counts were taken by CJ Hensch & Associates, Inc. on Thursday, October 6, 2016. Peak hours were reached between 7:30-8:30 AM for the AM peak hour, and 5:00-6:00 PM for the PM peak hour. The existing traffic volumes are shown in **Figure 2** and the raw traffic count data can be found in the **Appendix**.



LEGEND

→ LANE ASSIGNMENT

(TS) TRAFFIC SIGNAL

(SS) STOP SIGN

FIGURE 1
EXISTING LANE ASSIGNMENTS
AND TRAFFIC CONTROL



JONES CARTER

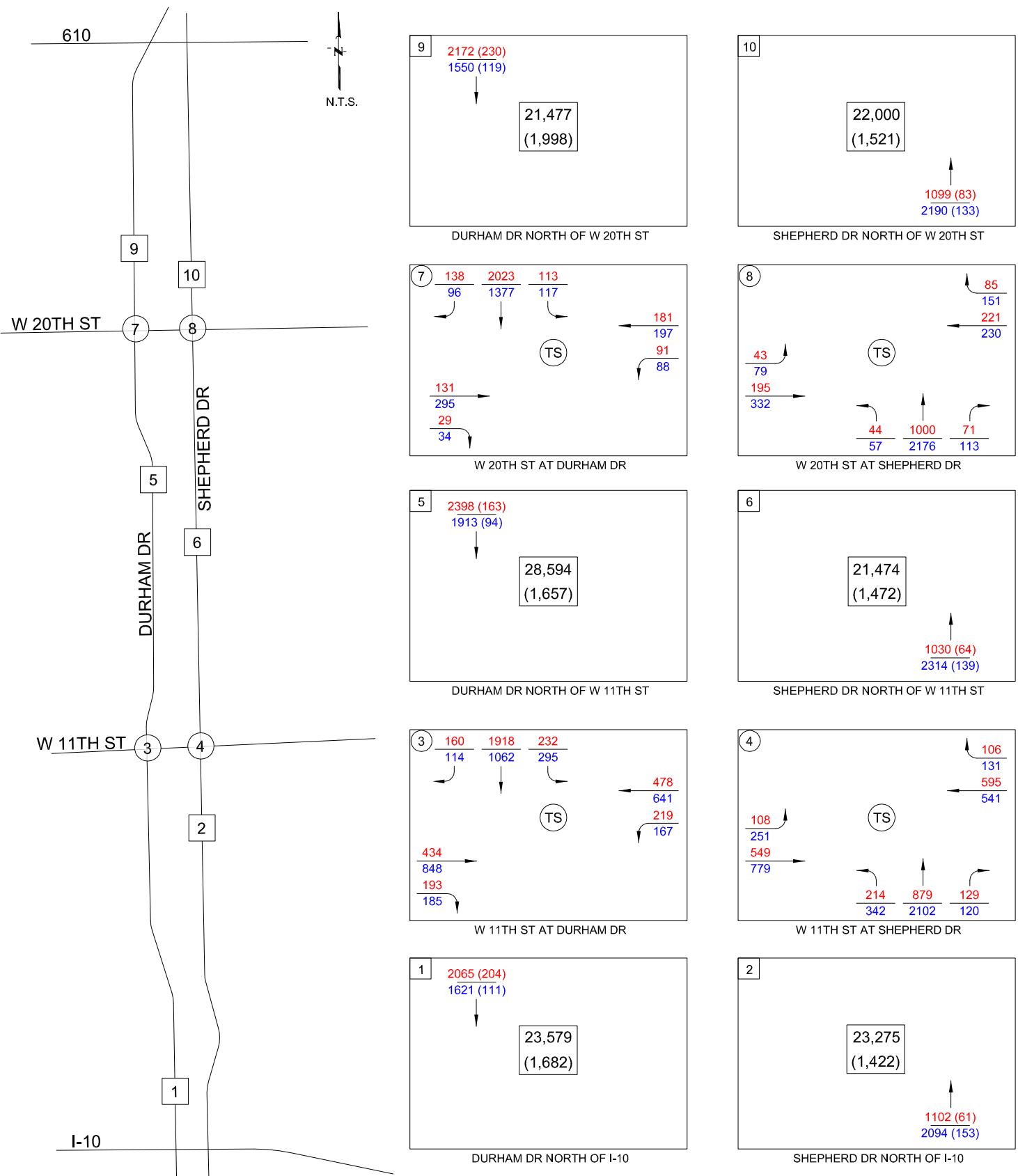


FIGURE 2
2016 EXISTING TRAFFIC VOLUMES
SHEPHERD DR AND DURHAM DR

Traffic Projections

Traffic projections were developed at the study area intersections and roadways for 2040 Projected Conditions based on the data collected and data provided by H-GAC.

Projected traffic volume data for the area roadways was received from the Houston Galveston Area Council (H-GAC) for 2018, 2035, and 2040. Annual growth rates for the roadway segments were calculated based on the percentage growth from the 2018 H-GAC traffic volumes to the 2035 H-GAC traffic volumes, as well as 2018 H-GAC traffic volumes to the 2040 H-GAC traffic volumes. The calculated annual growth rates were then applied to the 2016 Existing volumes to determine the projected 2040 traffic volumes. A growth rate of 0.7% per year for over 24 years was applied and projected volumes are shown in **Figure 3**. Annual Growth percentage calculations and H-GAC 2018 traffic volumes, H-GAC 2035 traffic volumes, and H-GAC 2040 traffic volumes are shown in **Table 1**.

Table 1 – Annual Growth Percentage Calculations

Roadway	Segment	ADT VOLUMES					% GROWTH PER YEAR (2018 TO 2040)
		2013 COH COUNTS	2016 COUNTS	2018 H-GAC PROJECTION	2035 H-GAC PROJECTION	2040 H-GAC PROJECTION	
Shepherd Drive	I-10 to 11 th	19,181	23,275	21,273	22,867	24,274	0.60%
	11 th to 20 th	19,575	21,474	16,713	17,655	18,005	0.34%
	20 th to 610	18,915	22,000	18,023	19,462	20,136	0.51%
Durham Drive	I-10 to 11 th	18,123	23,579	17,459	20,343	21,221	0.89%
	11 th to 20 th	22,196	28,594	11,599	13,389	14,222	0.93%
	20 th to 610	20,086	21,477	12,680	15,371	16,037	1.07%
						AVERAGE GROWTH %	0.7%

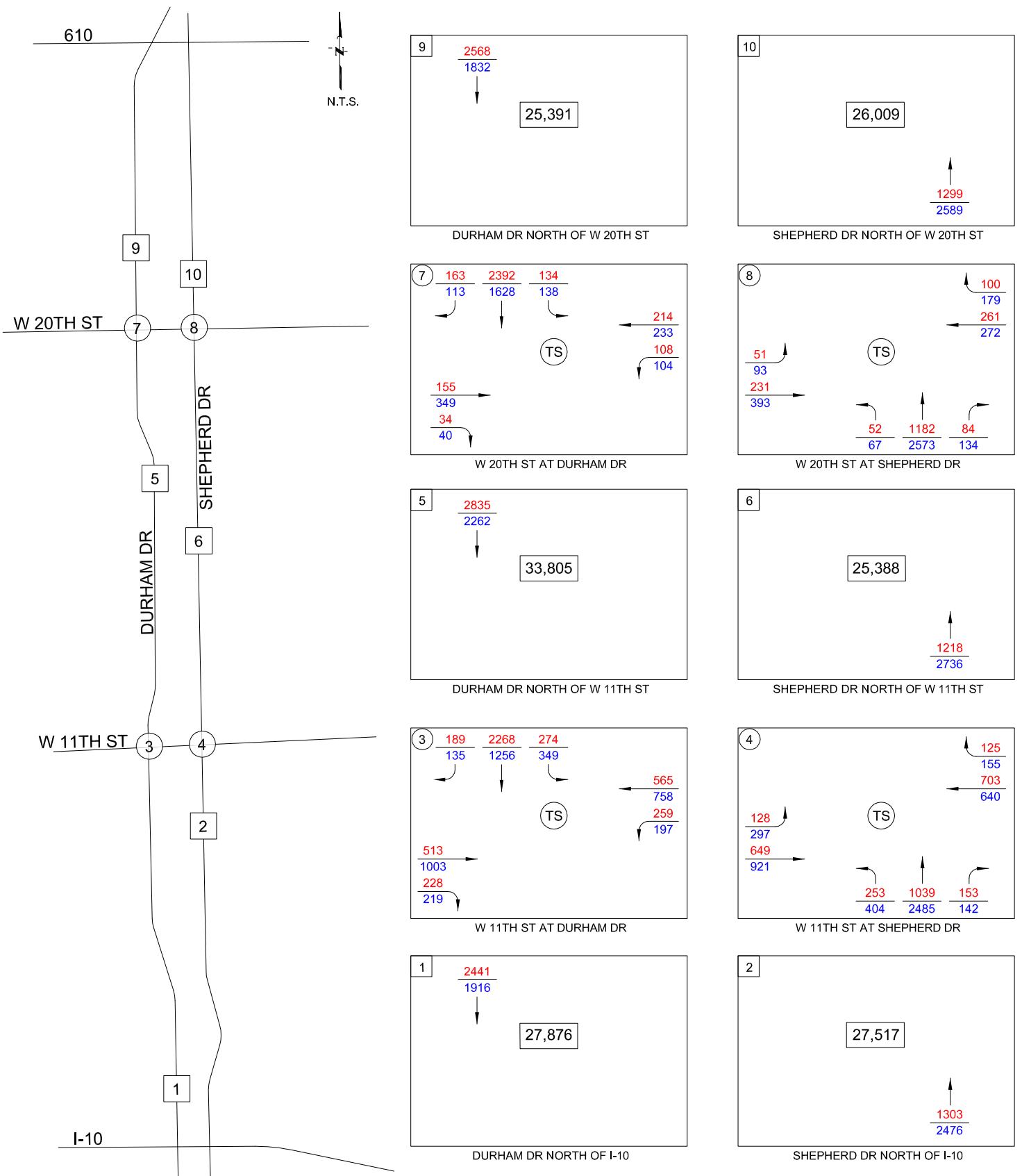


FIGURE 3
2040 PROJECTED TRAFFIC VOLUMES
SHEPHERD DR AND DURHAM DR

Roadway Capacity Analysis

Capacity Analysis was analyzed in comparing the 24 hour volumes per segment with the H-GAC defined capacity of an urban one-way street. The volumes on Shepherd Drive and Durham Drive and roadway capacities are shown for 2016 Existing Conditions in **Table 2** and shown for 2040 Projected Conditions in **Table 3**.

Table 2 – 2016 Existing Roadway Capacity

Roadway	Segment	24 HOUR VOLUMES	3-LANE ROADWAY CAPACITY	4-LANE ROADWAY CAPACITY
Shepherd Drive	IH-10 to 11 th	23,275	25,400	33,000
	11 th to 20 th	21,474	25,400	33,000
	20 th to IH 610	22,000	25,400	33,000
Durham Drive	IH-10 to 11 th	23,579	25,400	33,000
	11 th to 20 th	28,594	25,400	33,000
	20 th to IH 610	21,477	25,400	33,000

Table 3 – 2040 Projected Roadway Capacity

Roadway	Segment	24 HOUR VOLUMES	3-LANE ROADWAY CAPACITY	4-LANE ROADWAY CAPACITY
Shepherd Drive	IH-10 to 11 th	27,517	25,400	33,000
	11 th to 20 th	25,388	25,400	33,000
	20 th to IH 610	26,009	25,400	33,000
Durham Drive	IH-10 to 11 th	27,876	25,400	33,000
	11 th to 20 th	33,805	25,400	33,000
	20 th to IH 610	25,391	25,400	33,000

All segments of Shepherd Drive and Durham Drive are projected to be at or near the capacity of a 3-lane Roadway in 2040 with the exception of the segment of Durham Drive between 11th Street and 20th Street which is projected to be at the capacity of a 4-lane Roadway.

Intersection Capacity Analysis

Capacity Analysis was performed using the computer program *Synchro 9.1*, which is based on the procedures in the *Highway Capacity Manual*. Capacity Analysis provides information regarding traffic operations at an intersection and is expressed in terms of the level of service (LOS). The LOS indicates the average seconds of delay experienced by a motorist at a signalized intersection, at stop sign controlled approaches and left turn movements at an unsignalized intersection. Intersection LOS ranges from A to F, with LOS A representing free flow conditions and LOS F representing highly congested conditions. An intersection operating at or above LOS D is typically characterized by acceptable delays. The Level of Service Measurement and Qualitative Descriptions for Signalized and Unsignalized intersections are shown in **Table 4**.

Table 4 – Level of Service Measurement and Qualitative Descriptions

Level Of Service	Unsignalized	Signalized	
	Control Delay Per Vehicle (Sec)	Control Delay Per Vehicle (Sec)	Description
A	≤ 10	≤ 10	Good progression and short cycle lengths
B	> 10 and ≤ 15	> 10 and ≤ 20	Good progression or short cycle lengths, more vehicle stops
C	> 15 and ≤ 25	> 20 and ≤ 35	Fair progression and/or longer cycle lengths, some cycle failures
D	> 25 and ≤ 35	> 35 and ≤ 55	Congestion becomes noticeable, high volume to capacity ratio
E	> 35 and ≤ 50	> 55 and ≤ 80	Limit of acceptable delay, poor progression, long cycles, and/or high volume
F	> 50	> 80	Unacceptable to drivers, volume greater than capacity

Various improvement scenarios were evaluated at the study area intersections for 2040 Projected Conditions and are shown in **Figure 4**. The impact of the Scenario options at the study area intersections was analyzed using Capacity Analysis and the *Synchro 9.1* reports can be found in the **Appendix**.

Scenario 1

- 2040 Projected Traffic Volumes
- Maintain existing lane assignments

Scenario 2

- 2040 Projected Traffic Volumes
- Remove one northbound through lane on Shepherd Drive
- Remove one southbound through lane on Durham Drive

Scenario 3

- 2040 Projected Traffic Volumes
- West 20th Street
 - Remove one northbound through lane on Shepherd Drive
 - Remove one southbound through lane on Durham Drive
 - Install a westbound left turn lane at Durham Drive
- West 11th Street
 - Change southbound through/left-turn lane on Durham Drive to left-turn only
 - Install a southbound right turn lane on Durham Drive
 - Change northbound through/left-turn lane on Shepherd Drive to left-turn only
 - Install a westbound right turn lane on W 11th Street at Shepherd Drive
 - Install a northbound right turn lane on Shepherd Drive

Scenario 4

- 2040 Projected Traffic Volumes
- West 20th Street
 - Remove one northbound through lane on Shepherd Drive
 - Remove one southbound through lane on Durham Drive
 - Install a westbound left turn lane at Durham Drive
- West 11th Street
 - Change southbound through/left-turn lane on Durham Drive to left-turn only
 - Install a westbound double left turn lane on West 11th Street at Durham Drive
 - Change northbound through/left-turn lane on Shepherd Drive to left-turn only
 - Install a northbound right turn lane on Shepherd Drive
 - Install an eastbound double left turn lane on West 11th Street at Shepherd Drive

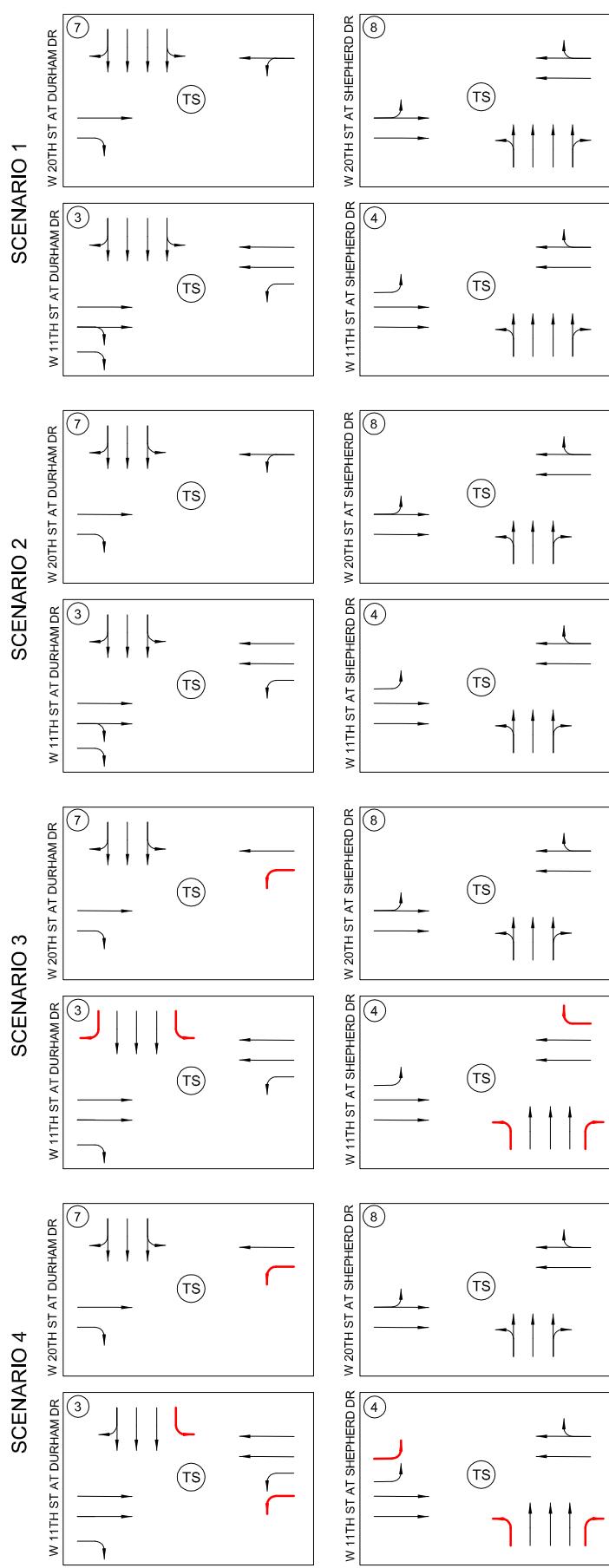
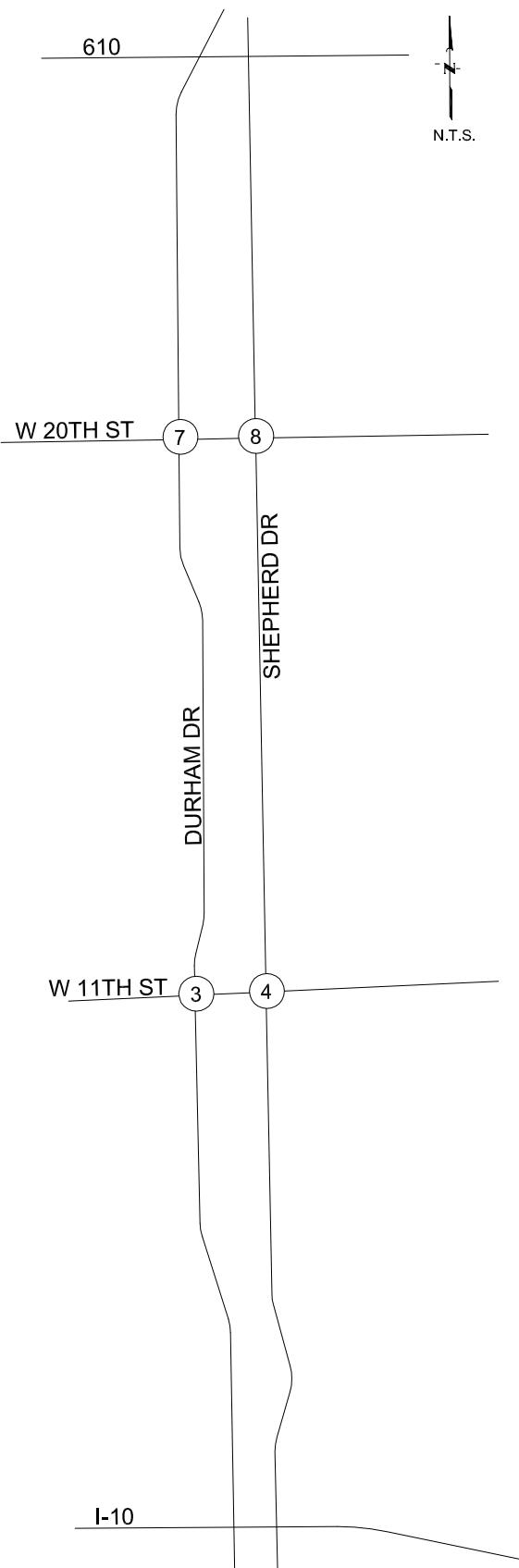


FIGURE 4
2040 PROJECTED INTERSECTION SCENARIOS

2016 Existing Conditions

Capacity analysis was performed for 2016 Existing Conditions for the Peak Hours at the study area intersections and is summarized in **Table 5**. All signalized intersections are projected to operate at an acceptable LOS during the AM and PM Peak hours for 2016 Existing Conditions with the exception of the following:

- Shepherd Drive at W 11th Street
 - LOS E in the AM Peak hour

Table 5 – Capacity Analysis – 2016 Existing Conditions

Intersection	AM Peak Hour (LOS (delay))					PM Peak Hour (LOS (delay))				
	EB	WB	NB	SB	Int	EB	WB	NB	SB	Int
Signalized Intersections										
Shepherd Drive at W 20 th St	C (27.5)	D (35.1)	A (9.3)		B (17.5)	D (39.0)	D (38.2)	A (2.2)		B (12.0)
Durham Drive at W 20 th St	C (32.5)	E (57.2)		B (14.0)	C (20.2)	C (20.3)	E (59.2)		B (10.9)	B (19.0)
Shepherd Drive at W 11 th St	B (15.0)	F (205.3)	B (15.7)		E (72.9)	C (31.4)	E (67.4)	D (35.5)		D (40.0)
Durham Drive at W 11 th St	D (46.3)	C (21.5)		D (36.9)	D (35.4)	D (36.7)	D (35.9)		C (26.9)	C (32.1)

Scenario 1

- 2040 Projected Traffic Volumes
- Maintain existing lane assignments

Capacity analysis was performed for 2040 Projected Conditions – Scenario 1 for the Peak Hours at the study area intersections and is summarized in **Table 6**. All signalized intersections are projected to operate at an acceptable LOS during the AM and PM Peak hours for 2040 Projected Conditions – Scenario 1 with the exception of the following:

- Shepherd Drive at W 11th Street
 - LOS E in the AM and PM Peak hours
- Durham Drive at W 11th Street
 - LOS F in the AM Peak hour

Table 6 – Capacity Analysis – 2040 Projected Conditions - Scenario 1

Intersection	AM Peak Hour (LOS (delay))					PM Peak Hour (LOS (delay))				
	EB	WB	NB	SB	Int	EB	WB	NB	SB	Int
Signalized Intersections*										
Shepherd Drive at W 20 th St	C (29.9)	D (37.3)	A (8.8)		B (18.1)	D (59.1)	D (36.7)	B (12.2)		C (20.9)
Durham Drive at W 20 th St	C (30.1)	E (58.5)		C (20.3)	C (25.3)	C (23.6)	F (97.4)		B (18.9)	C (30.2)
Shepherd Drive at W 11 th St	D (36.8)	F (146.4)	C (22.3)		E (63.4)	E (56.0)	F (121.0)	E (77.0)		E (79.7)
Durham Drive at W 11 th St	E (55.6)	D (40.6)		F (111.0)	F (87.3)	D (39.6)	C (29.1)		D (43.9)	D (38.9)

*Signal timing cycle lengths and split timings were optimized

Scenario 2

- 2040 Projected Traffic Volumes
- Remove one northbound through lane on Shepherd Drive
- Remove one southbound through lane on Durham Drive

Capacity analysis was performed for 2040 Projected Conditions – Scenario 2 for the Peak Hours at the study area intersections and is summarized in **Table 7**. All signalized intersections are projected to operate at an acceptable LOS during the AM and PM Peak hours for 2040 Projected Conditions – Scenario 2 with the exception of the following:

- Shepherd Drive at W 11th Street
 - LOS E in the AM Peak Hour and LOS F in the PM Peak hour
- Durham Drive at W 11th Street
 - LOS F in the AM Peak hour

Table 7 – Capacity Analysis – 2040 Projected Conditions - Scenario 2

Intersection	AM Peak Hour (LOS (delay))					PM Peak Hour (LOS (delay))				
	EB	WB	NB	SB	Int	EB	WB	NB	SB	Int
Signalized Intersections*										
Shepherd Drive at W 20 th St	D (45.4)	D (37.3)	A (9.8)		C (21.1)	E (62.6)	D (41.3)	C (26.8)		C (33.7)
Durham Drive at W 20 th St	C (34.4)	F (82.2)		D (36.2)	D (41.3)	C (23.3)	F (81.8)		C (23.9)	C (31.5)
Shepherd Drive at W 11 th St	D (36.8)	F (146.4)	C (25.4)		E (64.8)	E (76.0)	F (122.4)	F (164.5)		F (136.3)
Durham Drive at W 11 th St	E (55.7)	D (46.0)		F (209.5)	F (150.3)	D (53.0)	D (40.1)		D (53.3)	D (49.9)

*Signal timing cycle lengths and split timings were optimized

Scenario 3

- 2040 Projected Traffic Volumes
- West 20th Street
 - Remove one northbound through lane on Shepherd Drive
 - Remove one southbound through lane on Durham Drive
 - Install a westbound left turn lane at Durham Drive
- West 11th Street
 - Change southbound through/left-turn lane on Durham Drive to left-turn only
 - Install a southbound right turn lane on Durham Drive
 - Change northbound through/left-turn lane on Shepherd Drive to left-turn only
 - Install a westbound right turn lane on W 11th Street at Shepherd Drive
 - Install a northbound right turn lane on Shepherd Drive

Capacity analysis was performed for 2040 Projected Conditions – Scenario 3 for the Peak Hours at the study area intersections and is summarized in **Table 8**. All signalized intersections are projected to operate at an acceptable LOS during the AM and PM Peak hours for 2040 Projected Conditions – Scenario 3.

Table 8 – Capacity Analysis – 2040 Projected Conditions - Scenario 3

Intersection	AM Peak Hour (LOS (delay))					PM Peak Hour (LOS (delay))				
	EB	WB	NB	SB	Int	EB	WB	NB	SB	Int
Signalized Intersections*										
Shepherd Drive at W 20 th St	D (37.5)	D (42.7)	A (9.2)		C (20.6)	D (35.5)	D (36.7)	C (29.4)		C (31.2)
Durham Drive at W 20 th St	D (46.3)	D (52.7)		C (30.6)	C (34.2)	D (50.5)	D (42.3)		C (23.3)	C (30.5)
Shepherd Drive at W 11 th St	C (33.2)	D (48.7)	C (23.2)		C (33.4)	D (37.8)	D (53.8)	D (52.2)		D (49.0)
Durham Drive at W 11 th St	D (54.5)	D (44.3)		D (38.6)	D (42.5)	D (36.1)	B (16.8)		D (36.9)	C (31.7)

*Signal timing cycle lengths and split timings were optimized

Scenario 4

- 2040 Projected Traffic Volumes
- West 20th Street
 - Remove one northbound through lane on Shepherd Drive
 - Remove one southbound through lane on Durham Drive
 - Install a westbound left turn lane at Durham Drive
- West 11th Street
 - Change southbound through/left-turn lane on Durham Drive to left-turn only
 - Install a westbound double left turn lane on West 11th Street at Durham Drive
 - Change northbound through/left-turn lane on Shepherd Drive to left-turn only
 - Install a northbound right turn lane on Shepherd Drive
 - Install an eastbound double left turn lane on West 11th Street at Shepherd Drive

Capacity analysis was performed for 2040 Projected Conditions – Scenario 4 for the Peak Hours at the study area intersections and is summarized in **Table 9**. All signalized intersections are projected to operate at an acceptable LOS during the AM and PM Peak hours for 2040 Projected Conditions – Scenario 4.

Table 9 – Capacity Analysis – 2040 Projected Conditions - Scenario 4

Intersection	AM Peak Hour (LOS (delay))					PM Peak Hour (LOS (delay))				
	EB	WB	NB	SB	Int	EB	WB	NB	SB	Int
Signalized Intersections*										
Shepherd Drive at W 20 th St	D (45.4)	D (37.3)	A (9.8)		C (21.1)	D (35.5)	D (36.7)	C (29.4)		C (31.2)
Durham Drive at W 20 th St	D (46.3)	D (40.3)		C (30.6)	C (32.8)	D (50.5)	D (42.3)		C (23.3)	C (30.5)
Shepherd Drive at W 11 th St	C (31.5)	D (53.9)	C (23.3)		C (34.6)	D (49.0)	E (60.8)	D (54.6)		D (54.3)
Durham Drive at W 11 th St	D (54.9)	D (48.8)		D (48.3)	D (49.6)	D (54.8)	C (33.4)		C (29.4)	D (37.9)

*Signal timing cycle lengths and split timings were optimized

Recommendations

All segments of Shepherd Drive and Durham Drive are projected to be at or near the capacity of a 3-lane Roadway in 2040 with the exception of the segment of Durham Drive between 11th Street and 20th Street which is projected to be at the capacity of a 4-lane Roadway.

Scenario 4 includes the removal of one through lane on Shepherd Drive and Durham Drive as well as various intersection improvements at 11th Street and 20th Street. The signalized intersections of 11th Street and 20th Street at Shepherd Drive and Durham Drive are projected to operate at an acceptable intersection LOS in the AM and PM Peak hours for 2040 Projected Conditions with the improvements in Scenario 4. At 20th Street there is minimal ROW that can be acquired without impact to existing businesses/homes which limits the scope of potential intersection improvements.

Even though several segments of Shepherd Drive and Durham Drive are projected to be near or over capacity of a 3 lane roadway in Scenario 4 the recommended intersections improvements are projected to provide acceptable intersection LOS for 2040 Projected Conditions. It is recommended Scenario 4 be considered for construction.

Appendix

Contents

- Appendix A Traffic Counts
- Appendix B Annual Growth Calculations
- Appendix C Capacity Analysis – Existing Conditions
- Appendix D Capacity Analysis–2040 Projected Conditions–Scenario 1
- Appendix E Capacity Analysis–2040 Projected Conditions–Scenario 2
- Appendix F Capacity Analysis–2040 Projected Conditions–Scenario 3
- Appendix G Capacity Analysis–2040 Projected Conditions–Scenario 4

Appendix A

Traffic Counts

Northbound Shpeherd Dr between W 10th St and W 9th St

October 6, 2016

Mechanical Vehicle Classification

NORTHBOUND														
Time	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
0:00	29	0	23	6	0	0	0	0	0	0	0	0	0	0
0:15	34	0	29	5	0	0	0	0	0	0	0	0	0	0
0:30	26	0	24	2	0	0	0	0	0	0	0	0	0	0
0:45	24	0	19	5	0	0	0	0	0	0	0	0	0	0
Hr Total	113	0	95	18	0	0	0	0	0	0	0	0	0	0
1:00	15	0	13	2	0	0	0	0	0	0	0	0	0	0
1:15	17	1	14	1	0	0	1	0	0	0	0	0	0	0
1:30	12	0	10	1	0	1	0	0	0	0	0	0	0	0
1:45	4	0	4	0	0	0	0	0	0	0	0	0	0	0
Hr Total	48	1	41	4	0	1	1	0	0	0	0	0	0	0
2:00	4	0	4	0	0	0	0	0	0	0	0	0	0	0
2:15	16	0	15	0	0	0	1	0	0	0	0	0	0	0
2:30	8	0	7	1	0	0	0	0	0	0	0	0	0	0
2:45	8	0	8	0	0	0	0	0	0	0	0	0	0	0
Hr Total	36	0	34	1	0	0	1	0	0	0	0	0	0	0
3:00	7	0	5	2	0	0	0	0	0	0	0	0	0	0
3:15	12	0	11	1	0	0	0	0	0	0	0	0	0	0
3:30	8	0	5	1	1	0	0	0	0	1	0	0	0	0
3:45	2	1	1	0	0	0	0	0	0	0	0	0	0	0
Hr Total	29	1	22	4	1	0	0	0	0	1	0	0	0	0
4:00	6	0	5	1	0	0	0	0	0	0	0	0	0	0
4:15	8	0	7	1	0	0	0	0	0	0	0	0	0	0
4:30	13	0	9	4	0	0	0	0	0	0	0	0	0	0
4:45	15	2	10	0	0	2	0	0	1	0	0	0	0	0
Hr Total	42	2	31	6	0	2	0	0	1	0	0	0	0	0
5:00	24	0	15	5	0	2	1	0	0	1	0	0	0	0
5:15	16	0	15	0	0	0	1	0	0	0	0	0	0	0
5:30	35	0	22	9	1	2	0	0	0	1	0	0	0	0
5:45	63	4	49	6	1	0	0	0	1	2	0	0	0	0
Hr Total	138	4	101	20	2	4	2	0	1	4	0	0	0	0
6:00	78	0	54	17	1	4	0	0	1	1	0	0	0	0
6:15	92	0	66	18	1	3	0	0	2	2	0	0	0	0
6:30	121	1	80	30	3	6	1	0	0	0	0	0	0	0
6:45	144	0	118	18	3	4	0	0	1	0	0	0	0	0
Hr Total	435	1	318	83	8	17	1	0	4	3	0	0	0	0
7:00	181	4	122	40	2	6	5	0	2	0	0	0	0	0
7:15	216	2	159	39	6	7	0	0	0	2	0	1	0	0
7:30	271	2	194	56	3	11	2	0	3	0	0	0	0	0
7:45	284	1	203	68	2	7	1	0	0	2	0	0	0	0
Hr Total	952	9	678	203	13	31	8	0	5	4	0	1	0	0
8:00	276	1	187	64	8	11	3	0	2	0	0	0	0	0
8:15	271	2	197	46	7	13	3	1	2	0	0	0	0	0
8:30	271	0	199	43	6	17	2	0	3	1	0	0	0	0
8:45	273	8	178	74	8	5	0	0	0	0	0	0	0	0
Hr Total	1091	11	761	227	29	46	8	1	7	1	0	0	0	0
9:00	258	3	181	50	3	17	0	0	2	2	0	0	0	0
9:15	251	1	168	61	1	13	3	0	2	2	0	0	0	0
9:30	260	1	180	57	2	14	3	0	1	2	0	0	0	0
9:45	252	2	164	56	1	18	8	0	2	1	0	0	0	0
Hr Total	1021	7	693	224	7	62	14	0	7	7	0	0	0	0
10:00	265	3	172	67	6	13	0	1	3	0	0	0	0	0
10:15	291	2	193	67	2	17	5	0	4	1	0	0	0	0
10:30	237	0	151	59	3	21	1	0	2	0	0	0	0	0
10:45	263	2	188	47	5	16	1	0	4	0	0	0	0	0
Hr Total	1056	7	704	240	16	67	7	1	13	1	0	0	0	0

Northbound Shpeherd Dr between W 10th St and W 9th St

October 6, 2016

Mechanical Vehicle Classification

Time	NORTHBOUND													
	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
22:00	156	1	121	30	1	3	0	0	0	0	0	0	0	0
22:15	121	0	93	22	2	3	1	0	0	0	0	0	0	0
22:30	123	0	106	14	1	2	0	0	0	0	0	0	0	0
22:45	100	0	74	17	3	6	0	0	0	0	0	0	0	0
Hr Total	500	1	394	83	7	14	1	0	0	0	0	0	0	0
23:00	77	0	67	8	1	1	0	0	0	0	0	0	0	0
23:15	72	0	61	11	0	0	0	0	0	0	0	0	0	0
23:30	63	0	51	8	1	3	0	0	0	0	0	0	0	0
23:45	49	0	40	7	0	1	0	0	1	0	0	0	0	0
Hr Total	261	0	219	34	2	5	0	0	1	0	0	0	0	0
Gr. Total	23275	334	16986	4232	301	860	296	30	176	46	3	7	0	4
% of Total	100.0%	1.4%	73.0%	18.2%	1.3%	3.7%	1.3%	0.1%	0.8%	0.2%	0.0%	0.0%	0.0%	0.0%
	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
	NORTHBOUND													

Northbound Shpeherd Dr between W 15th St and W 14th St

October 6, 2016

Mechanical Vehicle Classification

NORTHBOUND														
Time	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
0:00	30	0	27	3	0	0	0	0	0	0	0	0	0	0
0:15	27	0	20	7	0	0	0	0	0	0	0	0	0	0
0:30	19	0	18	1	0	0	0	0	0	0	0	0	0	0
0:45	15	0	12	3	0	0	0	0	0	0	0	0	0	0
Hr Total	91	0	77	14	0	0	0	0	0	0	0	0	0	0
1:00	11	0	11	0	0	0	0	0	0	0	0	0	0	0
1:15	13	0	10	1	0	1	0	0	1	0	0	0	0	0
1:30	9	0	9	0	0	0	0	0	0	0	0	0	0	0
1:45	3	0	3	0	0	0	0	0	0	0	0	0	0	0
Hr Total	36	0	33	1	0	1	0	0	1	0	0	0	0	0
2:00	4	0	4	0	0	0	0	0	0	0	0	0	0	0
2:15	13	0	11	0	0	0	1	0	0	1	0	0	0	0
2:30	11	0	9	1	0	1	0	0	0	0	0	0	0	0
2:45	9	0	5	3	0	0	1	0	0	0	0	0	0	0
Hr Total	37	0	29	4	0	1	2	0	0	1	0	0	0	0
3:00	4	0	3	1	0	0	0	0	0	0	0	0	0	0
3:15	7	0	7	0	0	0	0	0	0	0	0	0	0	0
3:30	2	1	1	0	0	0	0	0	0	0	0	0	0	0
3:45	2	0	2	0	0	0	0	0	0	0	0	0	0	0
Hr Total	15	1	13	1	0	0	0	0	0	0	0	0	0	0
4:00	7	0	6	1	0	0	0	0	0	0	0	0	0	0
4:15	12	0	10	2	0	0	0	0	0	0	0	0	0	0
4:30	11	0	8	2	0	0	0	0	0	1	0	0	0	0
4:45	32	0	23	6	0	2	1	0	0	0	0	0	0	0
Hr Total	62	0	47	11	0	2	1	0	0	1	0	0	0	0
5:00	32	0	22	5	1	2	0	0	1	1	0	0	0	0
5:15	38	1	25	8	1	1	1	0	0	1	0	0	0	0
5:30	48	0	37	5	2	2	1	0	0	1	0	0	0	0
5:45	54	1	42	6	1	3	0	0	0	1	0	0	0	0
Hr Total	172	2	126	24	5	8	2	0	1	4	0	0	0	0
6:00	70	0	51	16	1	1	1	0	0	0	0	0	0	0
6:15	72	0	52	14	2	3	0	0	1	0	0	0	0	0
6:30	122	2	85	27	3	3	1	0	0	1	0	0	0	0
6:45	130	0	97	23	1	5	2	0	2	0	0	0	0	0
Hr Total	394	2	285	80	7	12	4	0	3	1	0	0	0	0
7:00	187	1	135	31	6	8	2	0	3	0	0	0	0	1
7:15	249	3	181	46	5	9	4	0	1	0	0	0	0	0
7:30	271	1	190	58	2	12	2	1	4	1	0	0	0	0
7:45	263	1	191	57	4	8	1	0	0	1	0	0	0	0
Hr Total	970	6	697	192	17	37	9	1	8	2	0	0	0	1
8:00	238	2	166	50	6	8	2	0	4	0	0	0	0	0
8:15	258	3	174	56	5	12	5	1	2	0	0	0	0	0
8:30	247	0	164	58	6	12	4	1	0	2	0	0	0	0
8:45	257	3	170	61	5	8	1	1	6	1	0	1	0	0
Hr Total	1000	8	674	225	22	40	12	3	12	3	0	1	0	0
9:00	243	2	153	62	1	16	4	0	5	0	0	0	0	0
9:15	225	0	144	63	1	12	4	0	0	1	0	0	0	0
9:30	248	4	160	59	3	16	2	1	2	1	0	0	0	0
9:45	250	1	150	60	5	20	9	1	2	1	0	1	0	0
Hr Total	966	7	607	244	10	64	19	2	9	3	0	1	0	0
10:00	235	0	145	69	1	13	3	0	3	1	0	0	0	0
10:15	225	2	142	61	2	13	2	1	2	0	0	0	0	0
10:30	204	2	124	55	2	15	3	0	2	0	1	0	0	0
10:45	251	1	159	71	2	11	2	0	3	2	0	0	0	0
Hr Total	915	5	570	256	7	52	10	1	10	3	1	0	0	0

Northbound Shpeherd Dr between W 15th St and W 14th St

October 6, 2016

Mechanical Vehicle Classification

Time	NORTHBOUND													
	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
22:00	111	2	88	16	1	4	0	0	0	0	0	0	0	0
22:15	127	0	93	27	1	5	1	0	0	0	0	0	0	0
22:30	87	0	70	16	0	0	1	0	0	0	0	0	0	0
22:45	89	1	65	16	1	5	0	1	0	0	0	0	0	0
Hr Total	414	3	316	75	3	14	2	1	0	0	0	0	0	0
23:00	76	1	59	15	0	0	0	0	1	0	0	0	0	0
23:15	55	0	44	8	0	2	1	0	0	0	0	0	0	0
23:30	48	0	41	2	1	3	1	0	0	0	0	0	0	0
23:45	43	0	32	10	0	1	0	0	0	0	0	0	0	0
Hr Total	222	1	176	35	1	6	2	0	0	1	0	0	0	0
Gr. Total	21474	155	15260	4412	175	812	337	35	223	42	5	15	2	1
% of Total	100.0%	0.7%	71.1%	20.5%	0.8%	3.8%	1.6%	0.2%	1.0%	0.2%	0.0%	0.1%	0.0%	0.0%
Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi	
NORTHBOUND														

Northbound Shepherd Dr between W 22nd St and W 21st St

October 6, 2016

Mechanical Vehicle Classification

Time	NORTHBOUND													
	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
22:00	130	2	96	27	1	3	1	0	0	0	0	0	0	0
22:15	119	0	92	25	0	2	0	0	0	0	0	0	0	0
22:30	109	1	87	17	1	2	1	0	0	0	0	0	0	0
22:45	90	0	71	15	0	4	0	0	0	0	0	0	0	0
Hr Total	448	3	346	84	2	11	2	0	0	0	0	0	0	0
23:00	75	0	59	12	1	2	0	0	0	1	0	0	0	0
23:15	65	0	50	15	0	0	0	0	0	0	0	0	0	0
23:30	39	0	32	4	1	2	0	0	0	0	0	0	0	0
23:45	45	0	35	6	0	3	0	0	0	1	0	0	0	0
Hr Total	224	0	176	37	2	7	0	0	0	2	0	0	0	0
Gr. Total	22000	91	15392	4802	194	868	279	27	262	56	3	12	2	12
% of Total	100.0%	0.4%	70.0%	21.8%	0.9%	3.9%	1.3%	0.1%	1.2%	0.3%	0.0%	0.1%	0.0%	0.1%
Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi	
NORTHBOUND														

Southbound Durham Dr between W 15th St and W 16th St

October 6, 2016
Mechanical Vehicle Classification

Time	SOUTHBOUND													
	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
22:00	79	0	64	13	0	2	0	0	0	0	0	0	0	0
22:15	97	2	77	15	1	2	0	0	0	0	0	0	0	0
22:30	68	0	57	11	0	0	0	0	0	0	0	0	0	0
22:45	62	0	47	12	1	2	0	0	0	0	0	0	0	0
Hr Total	306	2	245	51	2	6	0	0	0	0	0	0	0	0
23:00	64	0	50	13	0	1	0	0	0	0	0	0	0	0
23:15	37	0	29	6	1	1	0	0	0	0	0	0	0	0
23:30	28	0	20	6	0	1	0	0	0	1	0	0	0	0
23:45	48	0	40	8	0	0	0	0	0	0	0	0	0	0
Hr Total	177	0	139	33	1	3	0	0	0	1	0	0	0	0
Gr. Total	28594	1226	20357	5048	306	961	319	19	305	30	8	10	1	4
% of Total	100.0%	4.3%	71.2%	17.7%	1.1%	3.4%	1.1%	0.1%	1.1%	0.1%	0.0%	0.0%	0.0%	0.0%
Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi	
SOUTHBOUND														

Southbound Durham Dr between W 25th St and W 24th St

October 6, 2016

Mechanical Vehicle Classification

Time	SOUTHBOUND													
	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
22:00	72	1	55	12	0	4	0	0	0	0	0	0	0	0
22:15	74	0	52	16	1	5	0	0	0	0	0	0	0	0
22:30	63	0	44	18	0	1	0	0	0	0	0	0	0	0
22:45	51	0	39	8	1	3	0	0	0	0	0	0	0	0
Hr Total	260	1	190	54	2	13	0	0	0	0	0	0	0	0
23:00	43	1	30	10	0	1	1	0	0	0	0	0	0	0
23:15	39	0	32	5	1	1	0	0	0	0	0	0	0	0
23:30	30	0	16	12	1	1	0	0	0	0	0	0	0	0
23:45	43	1	32	8	0	1	0	0	1	0	0	0	0	0
Hr Total	155	2	110	35	2	4	1	0	1	0	0	0	0	0
Gr. Total	21477	258	14165	4880	176	1398	232	28	264	38	25	6	4	3
% of Total	100.0%	1.2%	66.0%	22.7%	0.8%	6.5%	1.1%	0.1%	1.2%	0.2%	0.1%	0.0%	0.0%	0.0%
	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
	SOUTHBOUND													

Southbound Durham Dr near LarkinSt

October 6, 2016

Mechanical Vehicle Classification

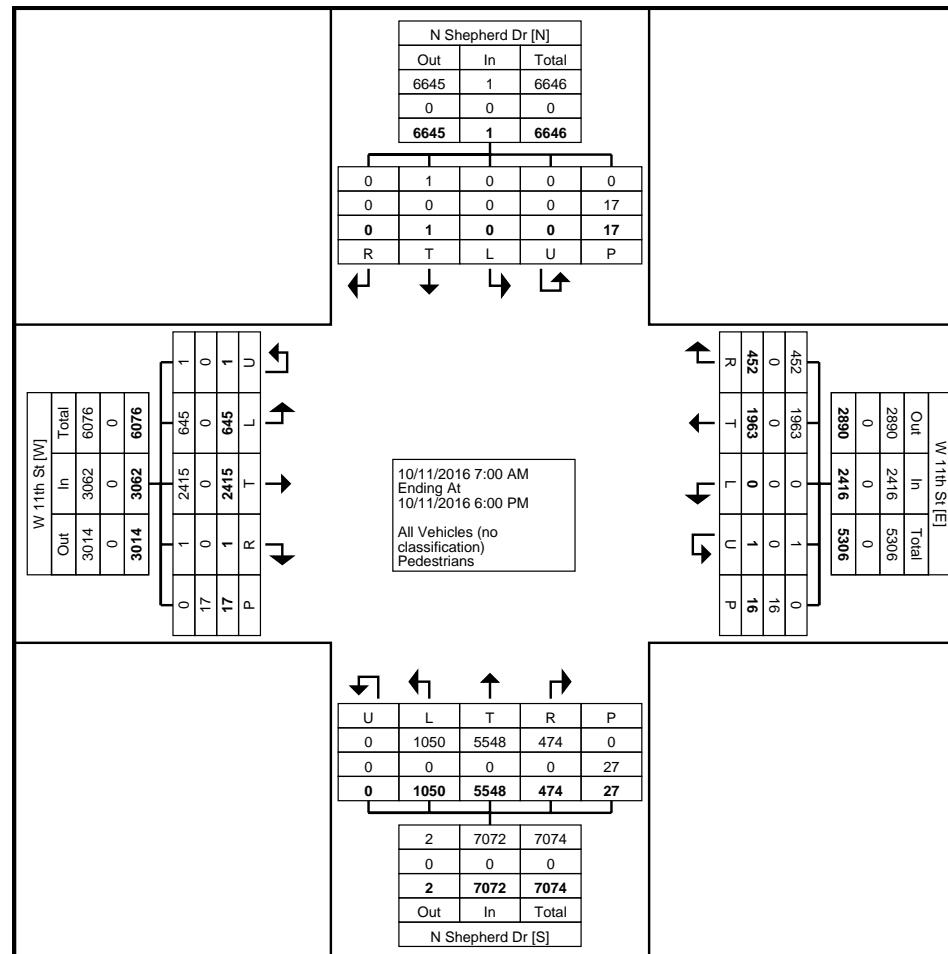
Time	SOUTHBOUND													
	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
22:00	149	0	119	26	1	3	0	0	0	0	0	0	0	0
22:15	103	0	82	14	1	4	1	0	1	0	0	0	0	0
22:30	101	1	84	13	1	2	0	0	0	0	0	0	0	0
22:45	72	0	57	14	0	1	0	0	0	0	0	0	0	0
Hr Total	425	1	342	67	3	10	1	0	1	0	0	0	0	0
23:00	53	0	44	7	1	1	0	0	0	0	0	0	0	0
23:15	54	0	42	9	1	2	0	0	0	0	0	0	0	0
23:30	28	0	21	4	0	2	0	0	0	1	0	0	0	0
23:45	40	3	34	3	0	0	0	0	0	0	0	0	0	0
Hr Total	175	3	141	23	2	5	0	0	0	1	0	0	0	0
Gr. Total	23579	322	16796	4595	184	1048	270	31	254	43	22	7	5	2
% of Total	100.0%	1.4%	71.2%	19.5%	0.8%	4.4%	1.1%	0.1%	1.1%	0.2%	0.1%	0.0%	0.0%	0.0%
	Total	Bikes	Cars & Trailers	2 Axle, Long	Buses	2 Axle, 6 Tire	3 Axle, Single	4 Axle, Single	<5 Axle, Double	5 Axle, Double	>6 Axle, Double	<6 Axle, Multi	6 Axle, Multi	>6 Axle, Multi
	SOUTHBOUND													



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Count Name: N Shepherd Dr at W 11th St
Site Code:
Start Date: 10/11/2016
Page No: 2



Turning Movement Data Plot



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Count Name: N Shepherd Dr at W 11th St
Site Code:
Start Date: 10/11/2016
Page No: 3

Turning Movement Peak Hour Data (7:30 AM)

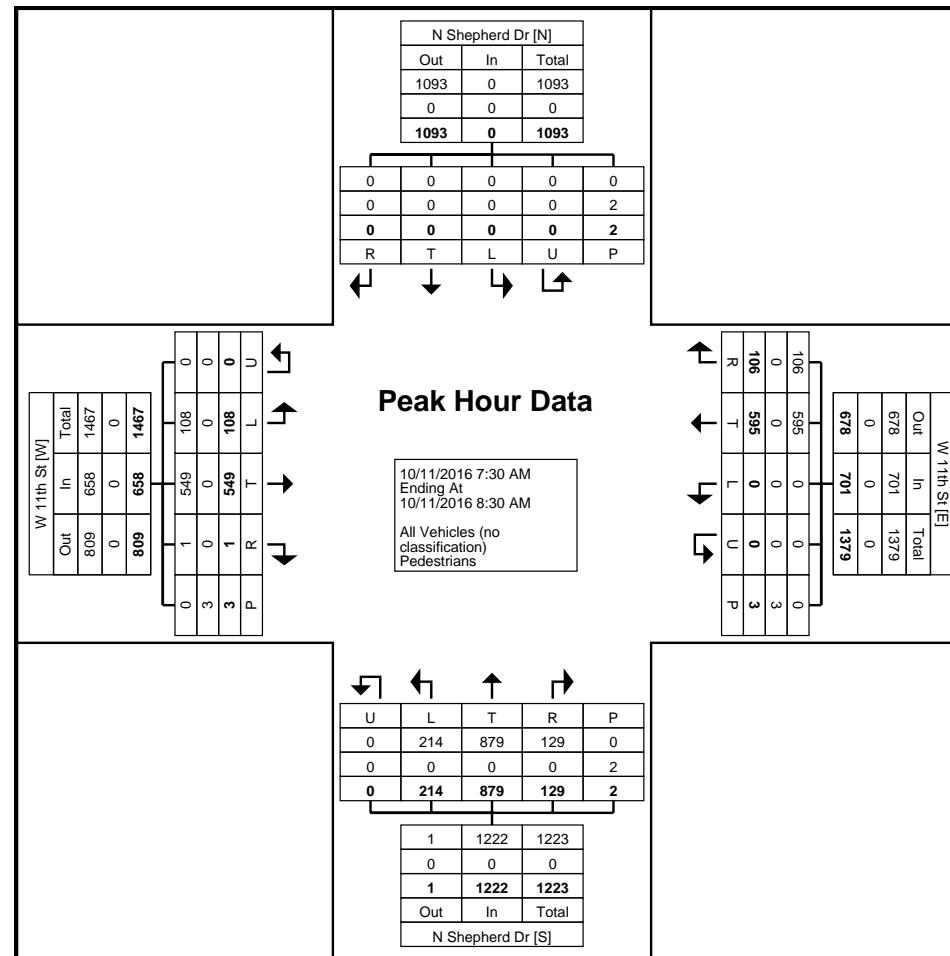
Start Time	N Shepherd Dr Southbound						W 11th St Westbound						N Shepherd Dr Northbound						W 11th St Eastbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	0	0	0	0	1	0	0	137	13	0	2	150	40	228	35	0	0	303	36	136	0	0	2	172	625
7:45 AM	0	0	0	0	1	0	0	188	30	0	1	218	59	217	28	0	1	304	28	148	0	0	1	176	698
8:00 AM	0	0	0	0	0	0	0	129	25	0	0	154	56	239	26	0	0	321	19	127	0	0	0	146	621
8:15 AM	0	0	0	0	0	0	0	141	38	0	0	179	59	195	40	0	1	294	25	138	1	0	0	164	637
Total	0	0	0	0	2	0	0	595	106	0	3	701	214	879	129	0	2	1222	108	549	1	0	3	658	2581
Approach %	NaN	NaN	NaN	NaN	-	-	0.0	84.9	15.1	0.0	-	-	17.5	71.9	10.6	0.0	-	-	16.4	83.4	0.2	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	23.1	4.1	0.0	-	27.2	8.3	34.1	5.0	0.0	-	47.3	4.2	21.3	0.0	0.0	-	25.5	-
PHF	0.000	0.000	0.000	0.000	-	0.000	0.000	0.791	0.697	0.000	-	0.804	0.907	0.919	0.806	0.000	-	0.952	0.750	0.927	0.250	0.000	-	0.935	0.924
All Vehicles (no classification)	0	0	0	0	-	0	0	595	106	0	-	701	214	879	129	0	-	1222	108	549	1	0	-	658	2581
% All Vehicles (no classification)	-	-	-	-	-	-	-	100.0	100.0	-	-	100.0	100.0	100.0	100.0	-	-	100.0	100.0	100.0	100.0	-	-	100.0	100.0
Pedestrians	-	-	-	-	-	2	-	-	-	-	3	-	-	-	-	-	2	-	-	-	-	-	3	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: N Shepherd Dr at W 11th St
Site Code:
Start Date: 10/11/2016
Page No: 4



Turning Movement Peak Hour Data Plot (7:30 AM)



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Count Name: N Shepherd Dr at W 11th St
Site Code:
Start Date: 10/11/2016
Page No: 5

Turning Movement Peak Hour Data (5:00 PM)

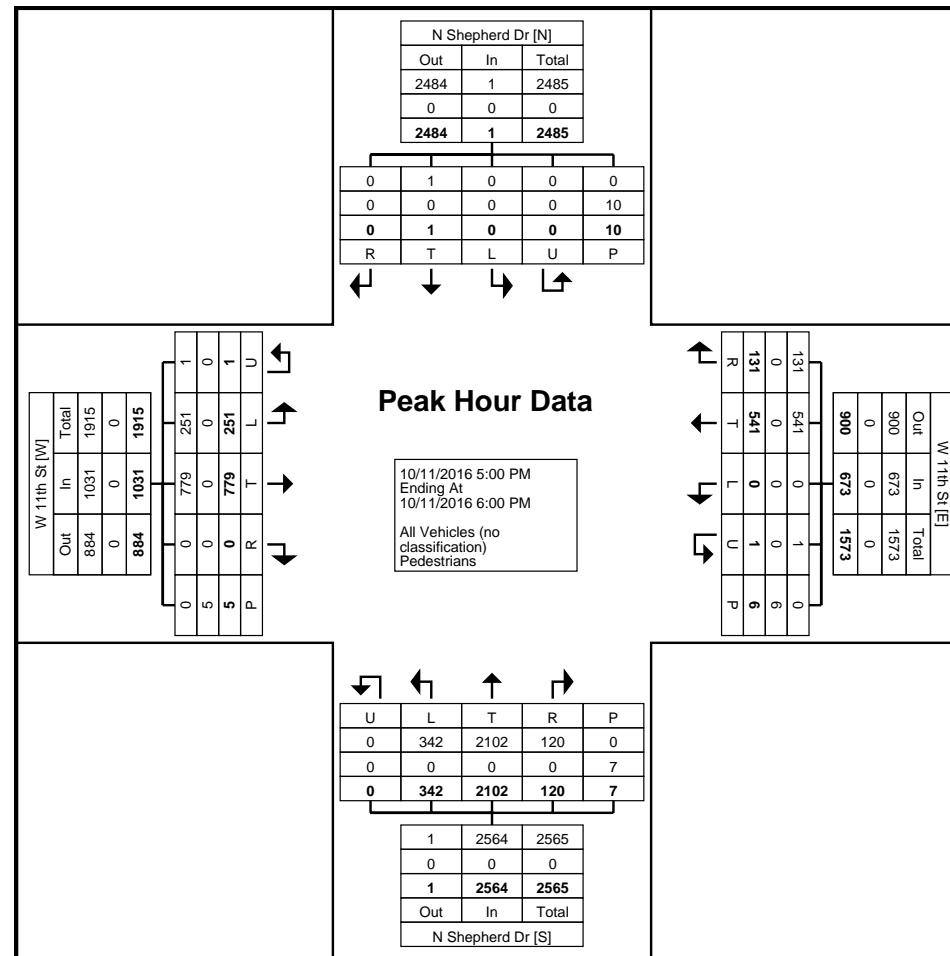
Start Time	N Shepherd Dr Southbound						W 11th St Westbound						N Shepherd Dr Northbound						W 11th St Eastbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
5:00 PM	0	1	0	0	6	1	0	114	20	1	1	135	81	568	37	0	1	686	64	205	0	0	2	269	1091
5:15 PM	0	0	0	0	0	0	0	162	32	0	2	194	85	468	29	0	2	582	64	211	0	1	2	276	1052
5:30 PM	0	0	0	0	1	0	0	127	42	0	2	169	90	565	30	0	2	685	66	183	0	0	1	249	1103
5:45 PM	0	0	0	0	3	0	0	138	37	0	1	175	86	501	24	0	2	611	57	180	0	0	0	237	1023
Total	0	1	0	0	10	1	0	541	131	1	6	673	342	2102	120	0	7	2564	251	779	0	1	5	1031	4269
Approach %	0.0	100.0	0.0	0.0	-	-	0.0	80.4	19.5	0.1	-	-	13.3	82.0	4.7	0.0	-	-	24.3	75.6	0.0	0.1	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	12.7	3.1	0.0	-	15.8	8.0	49.2	2.8	0.0	-	60.1	5.9	18.2	0.0	0.0	-	24.2	-
PHF	0.000	0.250	0.000	0.000	-	0.250	0.000	0.835	0.780	0.250	-	0.867	0.950	0.925	0.811	0.000	-	0.934	0.951	0.923	0.000	0.250	-	0.934	0.968
All Vehicles (no classification)	0	1	0	0	-	1	0	541	131	1	-	673	342	2102	120	0	-	2564	251	779	0	1	-	1031	4269
% All Vehicles (no classification)	-	100.0	-	-	-	100.0	-	100.0	100.0	100.0	-	100.0	100.0	100.0	100.0	-	-	100.0	100.0	100.0	-	100.0	-	100.0	100.0
Pedestrians	-	-	-	-	-	10	-	-	-	-	6	-	-	-	-	-	7	-	-	-	-	-	5	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: N Shepherd Dr at W 11th St
Site Code:
Start Date: 10/11/2016
Page No: 6



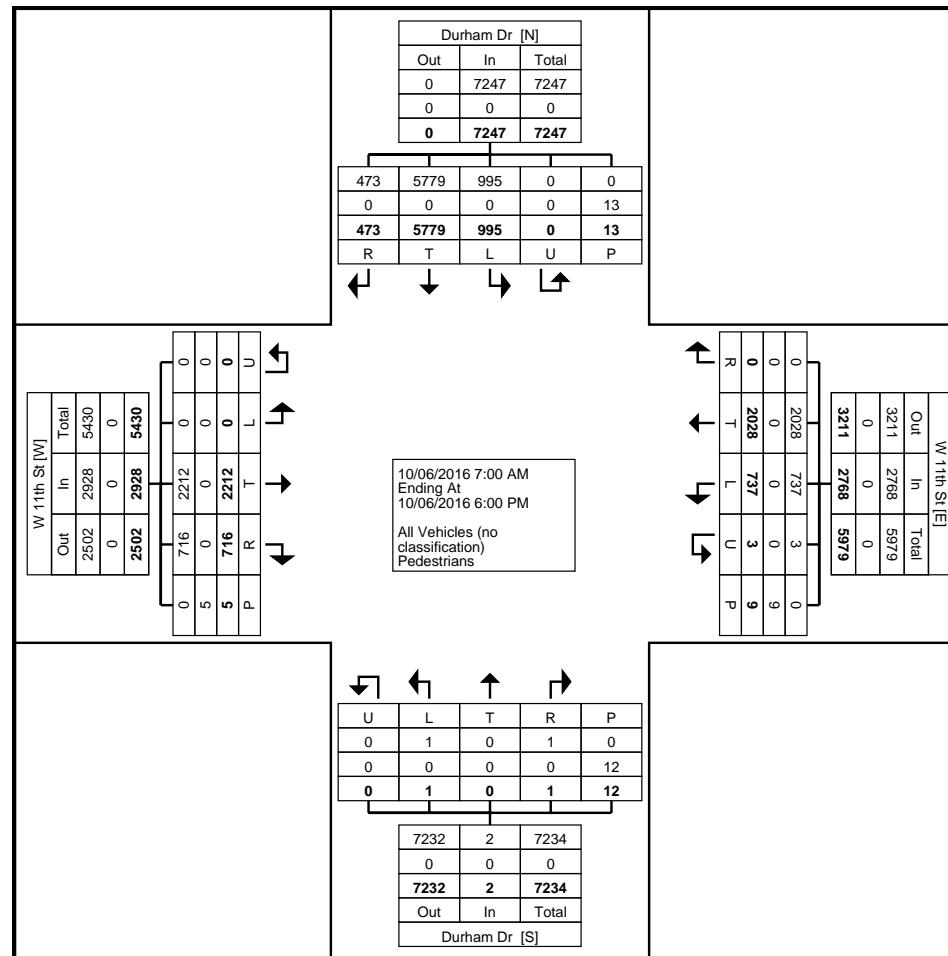
Turning Movement Peak Hour Data Plot (5:00 PM)



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Count Name: Durham Dr at W 11th St
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Page No: 2



Turning Movement Data Plot



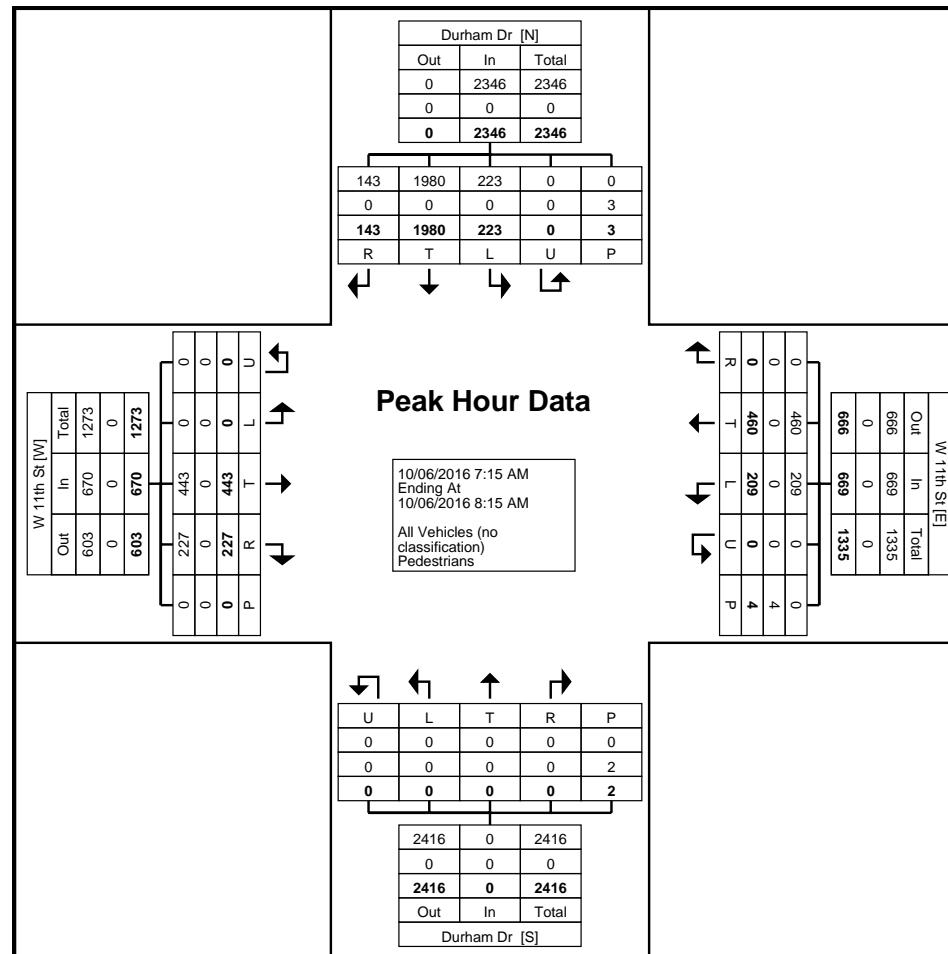
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Count Name: Durham Dr at W 11th St
Site Code:
Start Date: 10/06/2016
Page No: 3

Turning Movement Peak Hour Data (7:15 AM)

Start Time	Durham Dr Southbound						W 11th St Westbound						Durham Dr Northbound						W 11th St Eastbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:15 AM	53	566	28	0	1	647	42	78	0	0	2	120	0	0	0	0	0	0	0	105	61	0	0	166	933
7:30 AM	55	491	37	0	1	583	66	137	0	0	1	203	0	0	0	0	1	0	0	131	63	0	0	194	980
7:45 AM	58	472	51	0	0	581	48	132	0	0	0	180	0	0	0	0	1	0	0	98	57	0	0	155	916
8:00 AM	57	451	27	0	1	535	53	113	0	0	1	166	0	0	0	0	0	0	0	109	46	0	0	155	856
Total	223	1980	143	0	3	2346	209	460	0	0	4	669	0	0	0	0	2	0	0	443	227	0	0	670	3685
Approach %	9.5	84.4	6.1	0.0	-	-	31.2	68.8	0.0	0.0	-	-	NaN	NaN	NaN	NaN	-	-	0.0	66.1	33.9	0.0	-	-	-
Total %	6.1	53.7	3.9	0.0	-	63.7	5.7	12.5	0.0	0.0	-	18.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	12.0	6.2	0.0	-	18.2	-
PHF	0.961	0.875	0.701	0.000	-	0.906	0.792	0.839	0.000	0.000	-	0.824	0.000	0.000	0.000	0.000	-	0.000	0.000	0.845	0.901	0.000	-	0.863	0.940
All Vehicles (no classification)	223	1980	143	0	-	2346	209	460	0	0	-	669	0	0	0	0	-	0	0	443	227	0	-	670	3685
% All Vehicles (no classification)	100.0	100.0	100.0	-	-	100.0	100.0	100.0	-	-	-	100.0	-	-	-	-	-	-	100.0	100.0	-	-	100.0	100.0	
Pedestrians	-	-	-	-	-	3	-	-	-	-	-	4	-	-	-	-	-	2	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-



Turning Movement Peak Hour Data Plot (7:15 AM)



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Count Name: Durham Dr at W 11th St
Site Code:
Start Date: 10/06/2016
Page No: 5

Turning Movement Peak Hour Data (5:00 PM)

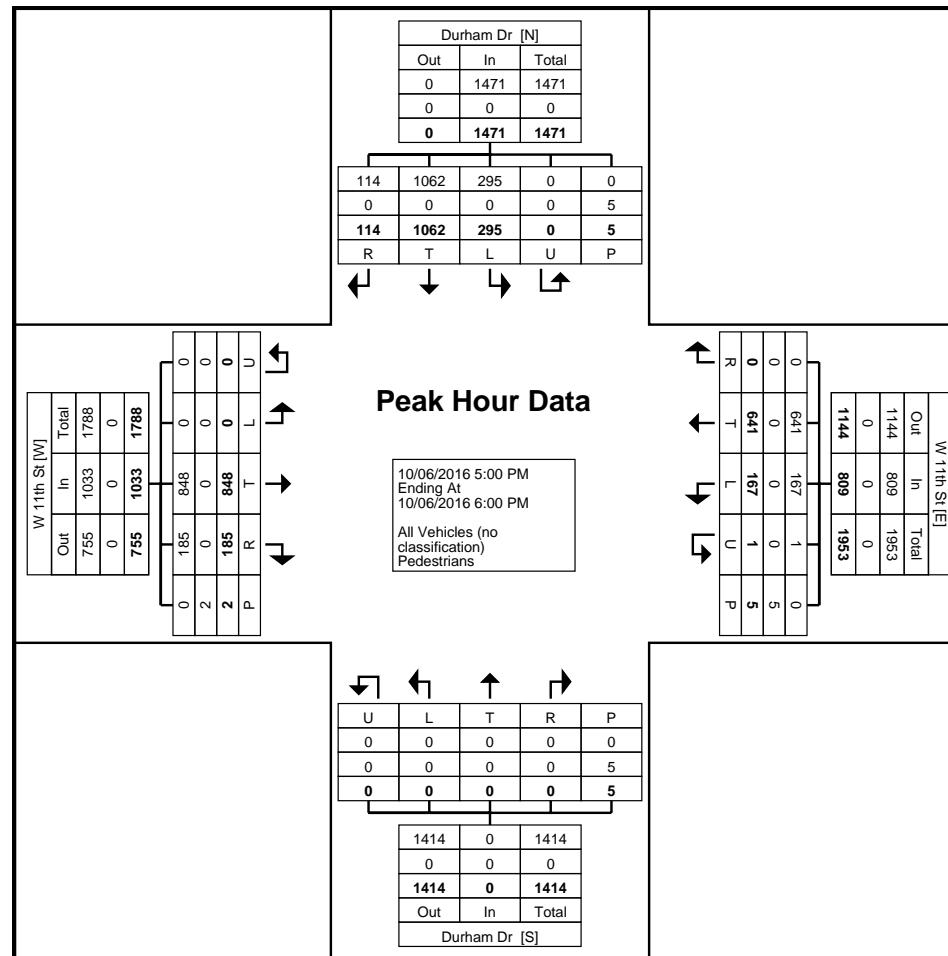
Start Time	Durham Dr Southbound						W 11th St Westbound						Durham Dr Northbound						W 11th St Eastbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
5:00 PM	71	234	32	0	1	337	45	156	0	1	0	202	0	0	0	0	2	0	0	220	46	0	0	266	805
5:15 PM	75	299	33	0	1	407	34	142	0	0	2	176	0	0	0	0	0	0	0	218	42	0	0	260	843
5:30 PM	64	248	25	0	3	337	42	184	0	0	3	226	0	0	0	0	2	0	0	198	43	0	1	241	804
5:45 PM	85	281	24	0	0	390	46	159	0	0	0	205	0	0	0	0	1	0	0	212	54	0	1	266	861
Total	295	1062	114	0	5	1471	167	641	0	1	5	809	0	0	0	0	5	0	0	848	185	0	2	1033	3313
Approach %	20.1	72.2	7.7	0.0	-	-	20.6	79.2	0.0	0.1	-	-	NaN	NaN	NaN	NaN	-	-	0.0	82.1	17.9	0.0	-	-	-
Total %	8.9	32.1	3.4	0.0	-	44.4	5.0	19.3	0.0	0.0	-	24.4	0.0	0.0	0.0	0.0	-	0.0	0.0	25.6	5.6	0.0	-	31.2	-
PHF	0.868	0.888	0.864	0.000	-	0.904	0.908	0.871	0.000	0.250	-	0.895	0.000	0.000	0.000	0.000	-	0.000	0.000	0.964	0.856	0.000	-	0.971	0.962
All Vehicles (no classification)	295	1062	114	0	-	1471	167	641	0	1	-	809	0	0	0	0	-	0	0	848	185	0	-	1033	3313
% All Vehicles (no classification)	100.0	100.0	100.0	-	-	100.0	100.0	100.0	-	100.0	-	100.0	-	-	-	-	-	-	100.0	100.0	-	-	100.0	100.0	
Pedestrians	-	-	-	-	-	5	-	-	-	-	5	-	-	-	-	-	5	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	



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Count Name: Durham Dr at W 11th St
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Start Date: 10/06/2016
Page No: 6



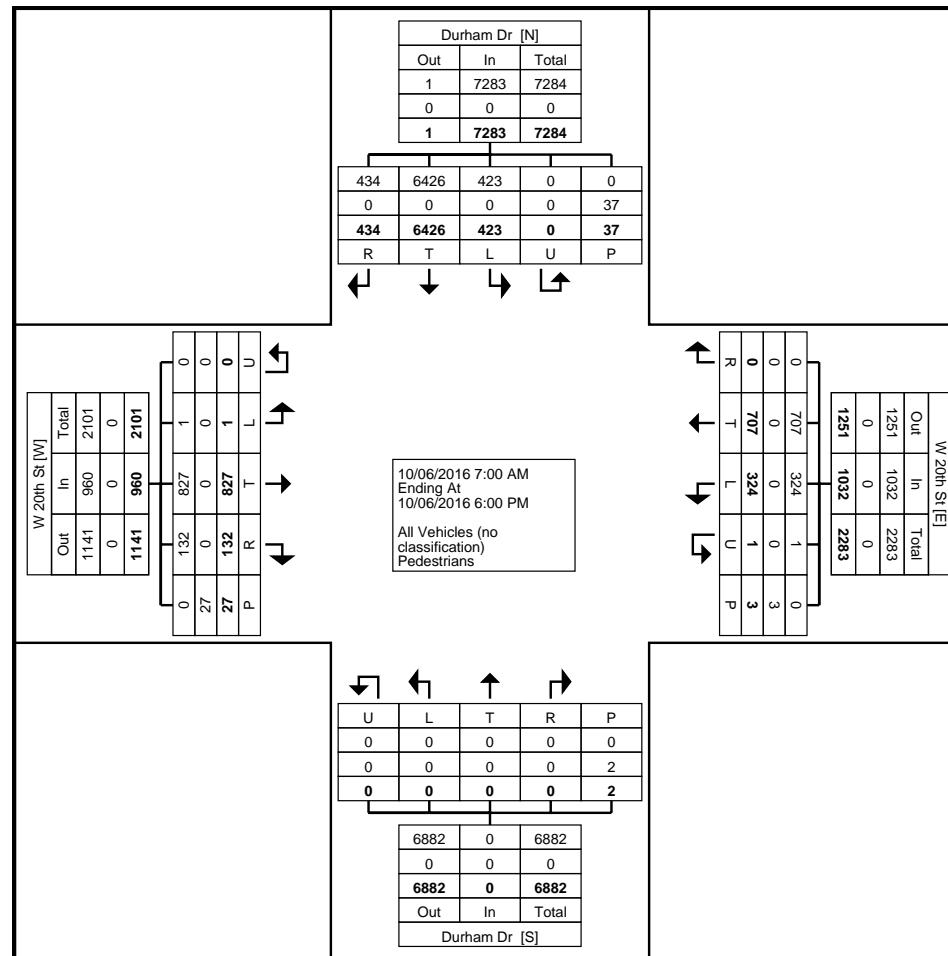
Turning Movement Peak Hour Data Plot (5:00 PM)



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Turning Movement Data Plot



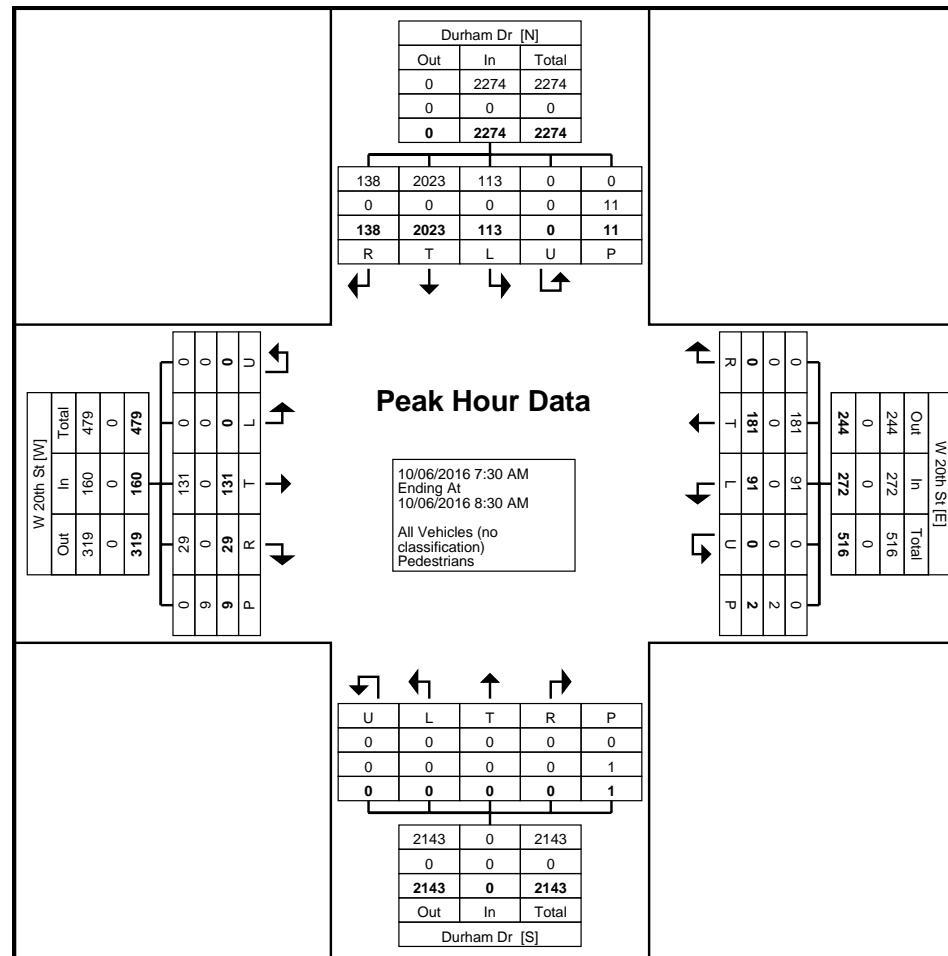
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Count Name: Durham Dr at W 20th St
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Page No: 3

Turning Movement Peak Hour Data (7:30 AM)

Start Time	Durham Dr Southbound						W 20th St Westbound						Durham Dr Northbound						W 20th St Eastbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	22	494	29	0	4	545	23	59	0	0	0	82	0	0	0	0	0	0	0	35	5	0	2	40	667
7:45 AM	28	538	41	0	5	607	20	46	0	0	1	66	0	0	0	0	1	0	0	44	9	0	3	53	726
8:00 AM	29	493	32	0	1	554	22	31	0	0	1	53	0	0	0	0	0	0	0	28	8	0	1	36	643
8:15 AM	34	498	36	0	1	568	26	45	0	0	0	71	0	0	0	0	0	0	0	24	7	0	3	31	670
Total	113	2023	138	0	11	2274	91	181	0	0	2	272	0	0	0	0	1	0	0	131	29	0	9	160	2706
Approach %	5.0	89.0	6.1	0.0	-	-	33.5	66.5	0.0	0.0	-	-	NaN	NaN	NaN	NaN	-	-	0.0	81.9	18.1	0.0	-	-	-
Total %	4.2	74.8	5.1	0.0	-	84.0	3.4	6.7	0.0	0.0	-	10.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	4.8	1.1	0.0	-	5.9	-
PHF	0.831	0.940	0.841	0.000	-	0.937	0.875	0.767	0.000	0.000	-	0.829	0.000	0.000	0.000	0.000	0.000	0.000	0.744	0.806	0.000	-	0.755	0.932	
All Vehicles (no classification)	113	2023	138	0	-	2274	91	181	0	0	-	272	0	0	0	0	-	0	0	131	29	0	-	160	2706
% All Vehicles (no classification)	100.0	100.0	100.0	-	-	100.0	100.0	100.0	-	-	-	100.0	-	-	-	-	-	-	100.0	100.0	-	-	100.0	100.0	
Pedestrians	-	-	-	-	-	11	-	-	-	-	-	2	-	-	-	-	-	1	-	-	-	-	9	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-



Turning Movement Peak Hour Data Plot (7:30 AM)



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Count Name: Durham Dr at W 20th St
Site Code:
Start Date: 10/06/2016
Page No: 5

Turning Movement Peak Hour Data (5:00 PM)

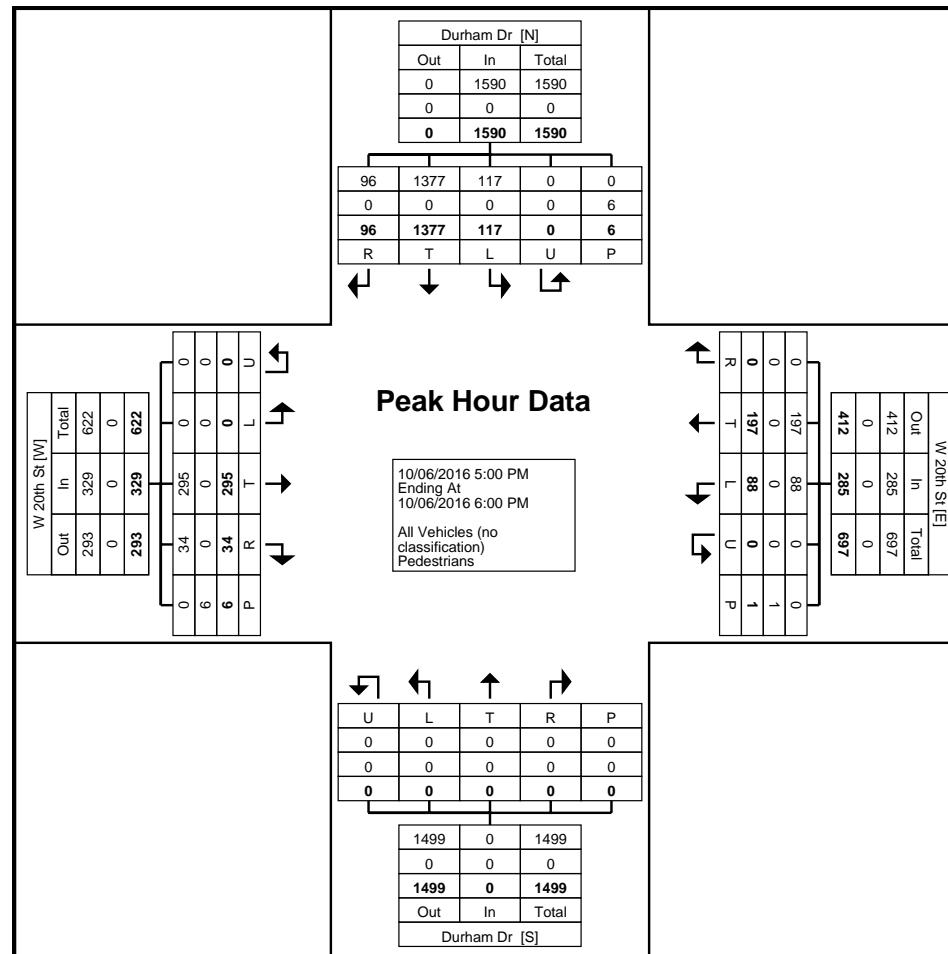
Start Time	Durham Dr Southbound						W 20th St Westbound						Durham Dr Northbound						W 20th St Eastbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
5:00 PM	22	354	22	0	3	398	21	51	0	0	0	72	0	0	0	0	0	0	0	72	8	0	2	80	550
5:15 PM	24	344	25	0	0	393	25	42	0	0	1	67	0	0	0	0	0	0	0	95	9	0	2	104	564
5:30 PM	33	334	20	0	1	387	24	48	0	0	0	72	0	0	0	0	0	0	0	69	9	0	1	78	537
5:45 PM	38	345	29	0	2	412	18	56	0	0	0	74	0	0	0	0	0	0	0	59	8	0	1	67	553
Total	117	1377	96	0	6	1590	88	197	0	0	1	285	0	0	0	0	0	0	0	295	34	0	6	329	2204
Approach %	7.4	86.6	6.0	0.0	-	-	30.9	69.1	0.0	0.0	-	-	NaN	NaN	NaN	NaN	-	-	0.0	89.7	10.3	0.0	-	-	-
Total %	5.3	62.5	4.4	0.0	-	72.1	4.0	8.9	0.0	0.0	-	12.9	0.0	0.0	0.0	0.0	-	0.0	0.0	13.4	1.5	0.0	-	14.9	-
PHF	0.770	0.972	0.828	0.000	-	0.965	0.880	0.879	0.000	0.000	-	0.963	0.000	0.000	0.000	0.000	-	0.000	0.776	0.944	0.000	-	0.791	0.977	
All Vehicles (no classification)	117	1377	96	0	-	1590	88	197	0	0	-	285	0	0	0	0	-	0	0	295	34	0	-	329	2204
% All Vehicles (no classification)	100.0	100.0	100.0	-	-	100.0	100.0	100.0	-	-	-	100.0	-	-	-	-	-	-	100.0	100.0	-	-	100.0	100.0	
Pedestrians	-	-	-	-	-	6	-	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	6	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	100.0	-	-



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Count Name: Durham Dr at W 20th St
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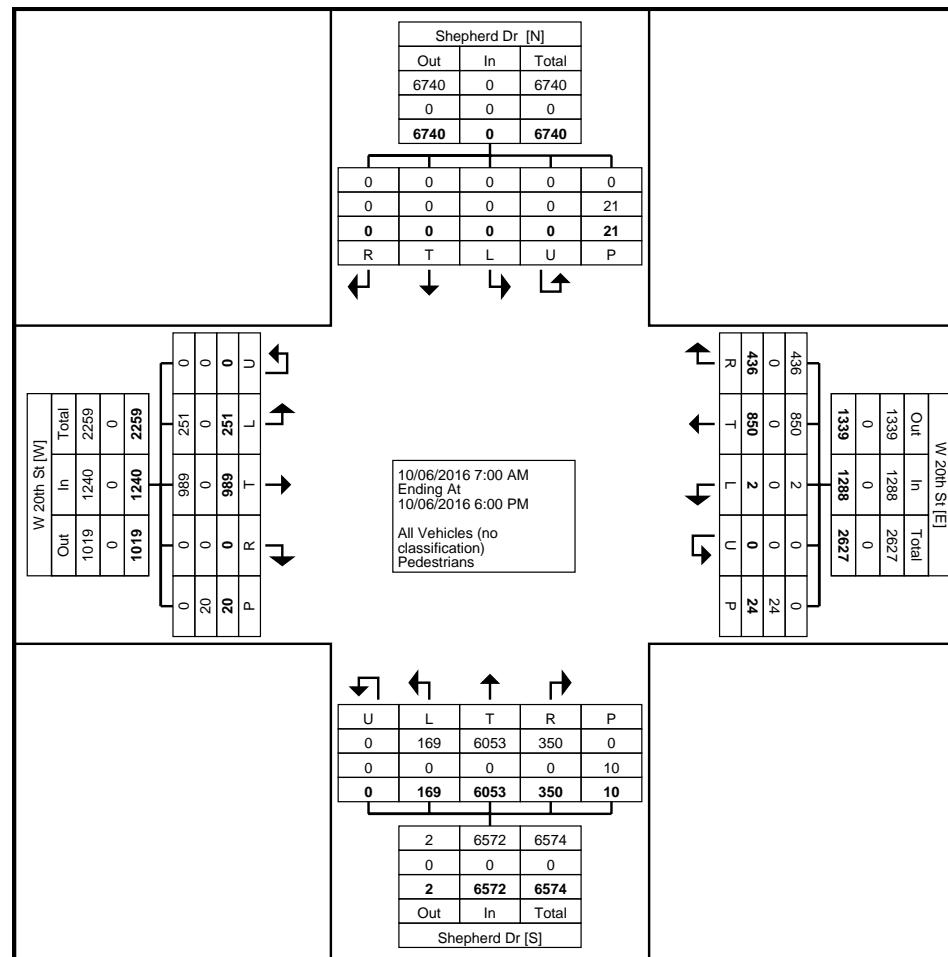


Turning Movement Peak Hour Data Plot (5:00 PM)

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Count Name: Shepherd Dr at W 20th St
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Turning Movement Data Plot



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Count Name: Shepherd Dr at W 20th St
Site Code:
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Page No: 3

Turning Movement Peak Hour Data (7:30 AM)

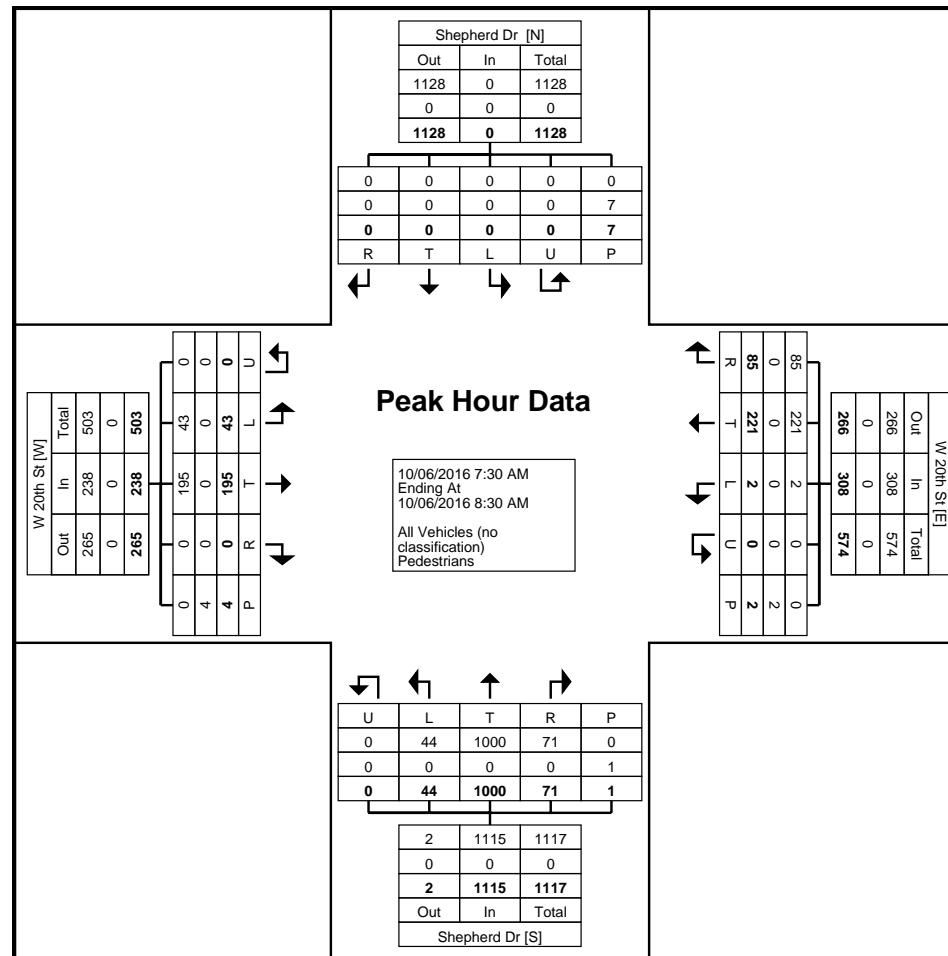
Start Time	Shepherd Dr Southbound						W 20th St Westbound						Shepherd Dr Northbound						W 20th St Eastbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	0	0	0	0	4	0	0	72	18	0	2	90	8	262	15	0	0	285	16	44	0	0	1	60	435
7:45 AM	0	0	0	0	2	0	0	51	18	0	0	69	9	252	22	0	0	283	9	59	0	0	0	68	420
8:00 AM	0	0	0	0	1	0	2	46	23	0	0	71	12	223	14	0	1	249	9	49	0	0	1	58	378
8:15 AM	0	0	0	0	0	0	0	52	26	0	0	78	15	263	20	0	0	298	9	43	0	0	2	52	428
Total	0	0	0	0	7	0	2	221	85	0	2	308	44	1000	71	0	1	1115	43	195	0	0	4	238	1661
Approach %	NaN	NaN	NaN	NaN	-	-	0.6	71.8	27.6	0.0	-	-	3.9	89.7	6.4	0.0	-	-	18.1	81.9	0.0	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.1	13.3	5.1	0.0	-	18.5	2.6	60.2	4.3	0.0	-	67.1	2.6	11.7	0.0	0.0	-	14.3	-
PHF	0.000	0.000	0.000	0.000	-	0.000	0.250	0.767	0.817	0.000	-	0.856	0.733	0.951	0.807	0.000	-	0.935	0.672	0.826	0.000	0.000	-	0.875	0.955
All Vehicles (no classification)	0	0	0	0	-	0	2	221	85	0	-	308	44	1000	71	0	-	1115	43	195	0	0	-	238	1661
% All Vehicles (no classification)	-	-	-	-	-	-	100.0	100.0	100.0	-	-	100.0	100.0	100.0	100.0	-	-	100.0	100.0	100.0	-	-	-	100.0	100.0
Pedestrians	-	-	-	-	-	7	-	-	-	-	2	-	-	-	-	-	1	-	-	-	-	-	4	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: Shepherd Dr at W 20th St
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Start Date: 10/06/2016
Page No: 4



Turning Movement Peak Hour Data Plot (7:30 AM)



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Count Name: Shepherd Dr at W 20th St
Site Code:
Start Date: 10/06/2016
Page No: 5

Turning Movement Peak Hour Data (5:00 PM)

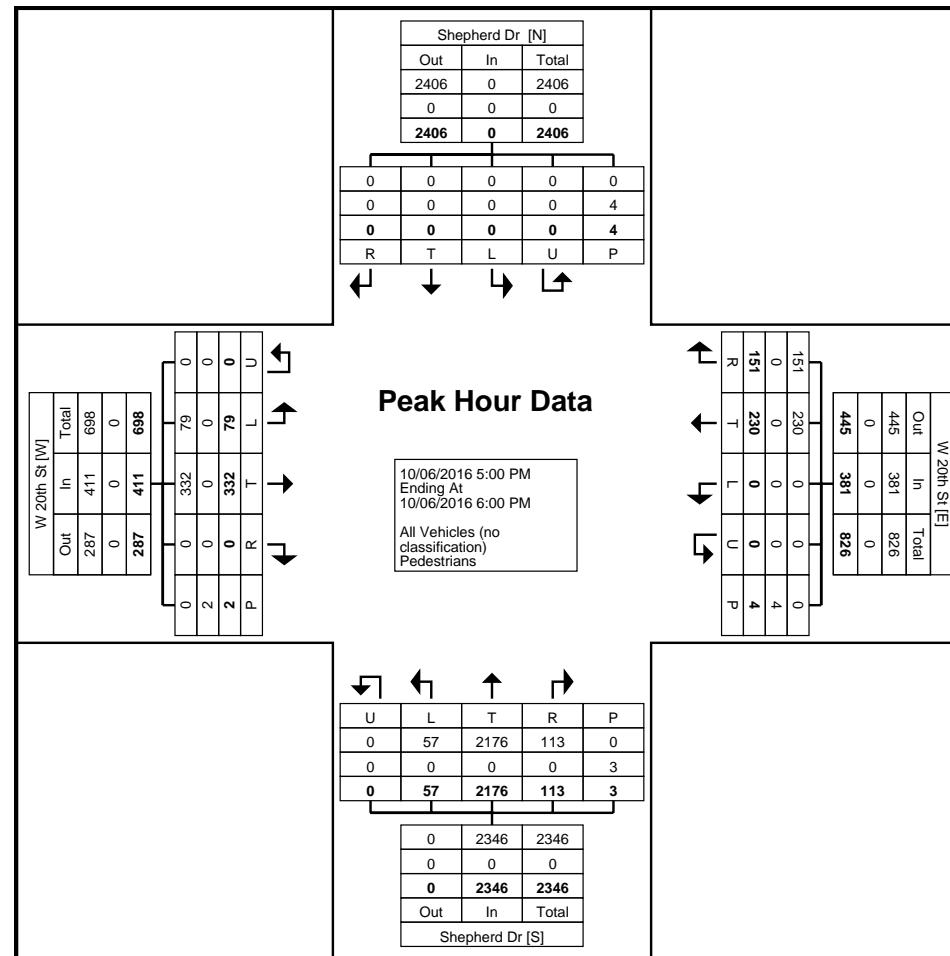
Start Time	Shepherd Dr Southbound						W 20th St Westbound						Shepherd Dr Northbound						W 20th St Eastbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
5:00 PM	0	0	0	0	4	0	0	66	43	0	1	109	14	535	16	0	1	565	23	73	0	0	0	96	770
5:15 PM	0	0	0	0	0	0	0	57	41	0	1	98	10	559	35	0	0	604	16	98	0	0	0	114	816
5:30 PM	0	0	0	0	0	0	0	54	38	0	0	92	13	516	27	0	1	556	21	84	0	0	1	105	753
5:45 PM	0	0	0	0	0	0	0	53	29	0	2	82	20	566	35	0	1	621	19	77	0	0	1	96	799
Total	0	0	0	0	4	0	0	230	151	0	4	381	57	2176	113	0	3	2346	79	332	0	0	2	411	3138
Approach %	NaN	NaN	NaN	NaN	-	-	0.0	60.4	39.6	0.0	-	-	2.4	92.8	4.8	0.0	-	-	19.2	80.8	0.0	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	7.3	4.8	0.0	-	12.1	1.8	69.3	3.6	0.0	-	74.8	2.5	10.6	0.0	0.0	-	13.1	-
PHF	0.000	0.000	0.000	0.000	-	0.000	0.000	0.871	0.878	0.000	-	0.874	0.713	0.961	0.807	0.000	-	0.944	0.859	0.847	0.000	0.000	-	0.901	0.961
All Vehicles (no classification)	0	0	0	0	-	0	0	230	151	0	-	381	57	2176	113	0	-	2346	79	332	0	0	-	411	3138
% All Vehicles (no classification)	-	-	-	-	-	-	-	100.0	100.0	-	-	100.0	100.0	100.0	100.0	-	-	100.0	100.0	100.0	-	-	-	100.0	100.0
Pedestrians	-	-	-	-	-	4	-	-	-	-	4	-	-	-	-	-	-	3	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-



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Pasadena, Texas, United States 77503
281-487-5417 cwright@jonescarter.com

Count Name: Shepherd Dr at W 20th St
Site Code:
Start Date: 10/06/2016
Page No: 6



Turning Movement Peak Hour Data Plot (5:00 PM)

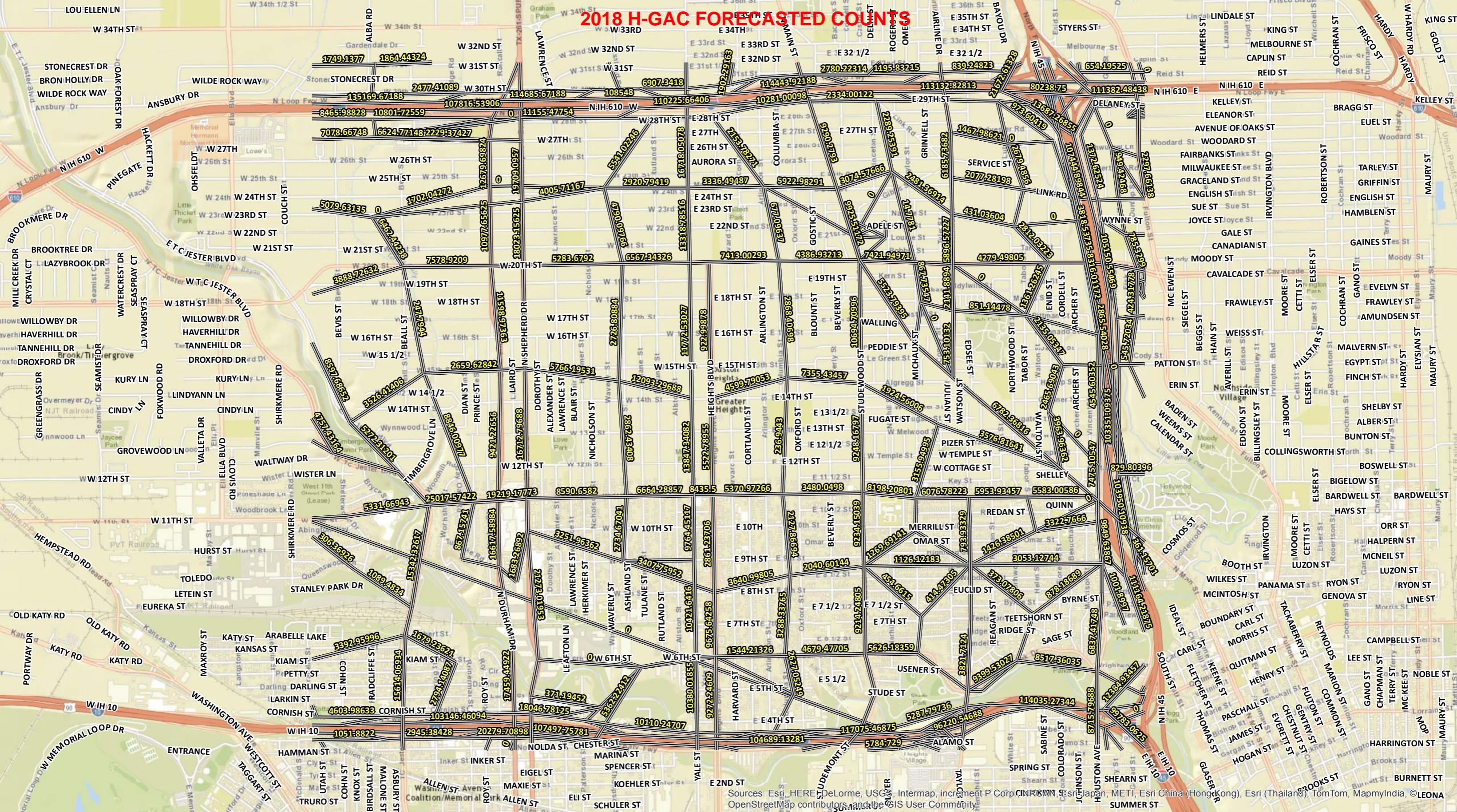
Appendix B

Annual Growth Calculations

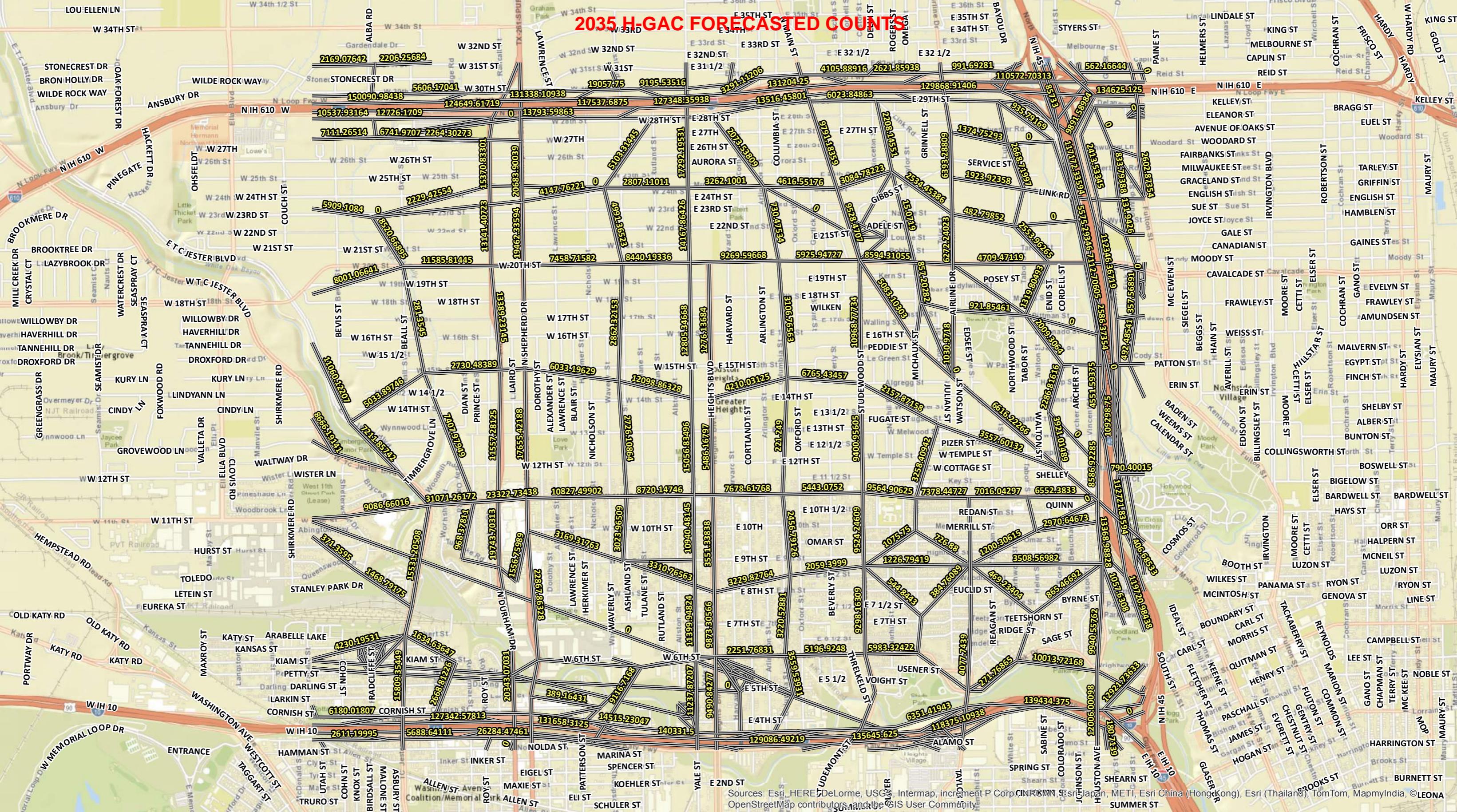
TRAFFIC GROWTH CALCULATIONS ON SHEPHERD DR AND DURHAM DR

Locations		ADT Volumes			% Growth	
		2016 CJ HENSCH COUNTS	2018 H-GAC COUNTS	2040 H-GAC COUNTS	2016 to 2018	2016 to 2040
N of 20th Shepherd Dr		22000	18023	20136	-9.5%	-0.4%
N of 20th Durham Dr		21477	12680	16037	-23.2%	-1.2%
N of 11th Shepherd Dr		21474	16713	18005	-11.8%	-0.7%
N of 11th Durham Dr		28594	11599	14222	-36.3%	-2.9%
S of 11th Shepherd Dr		23275	21273	24274	-4.4%	0.2%
S of 11th Durham Dr		23579	17459	21221	-14.0%	-0.4%

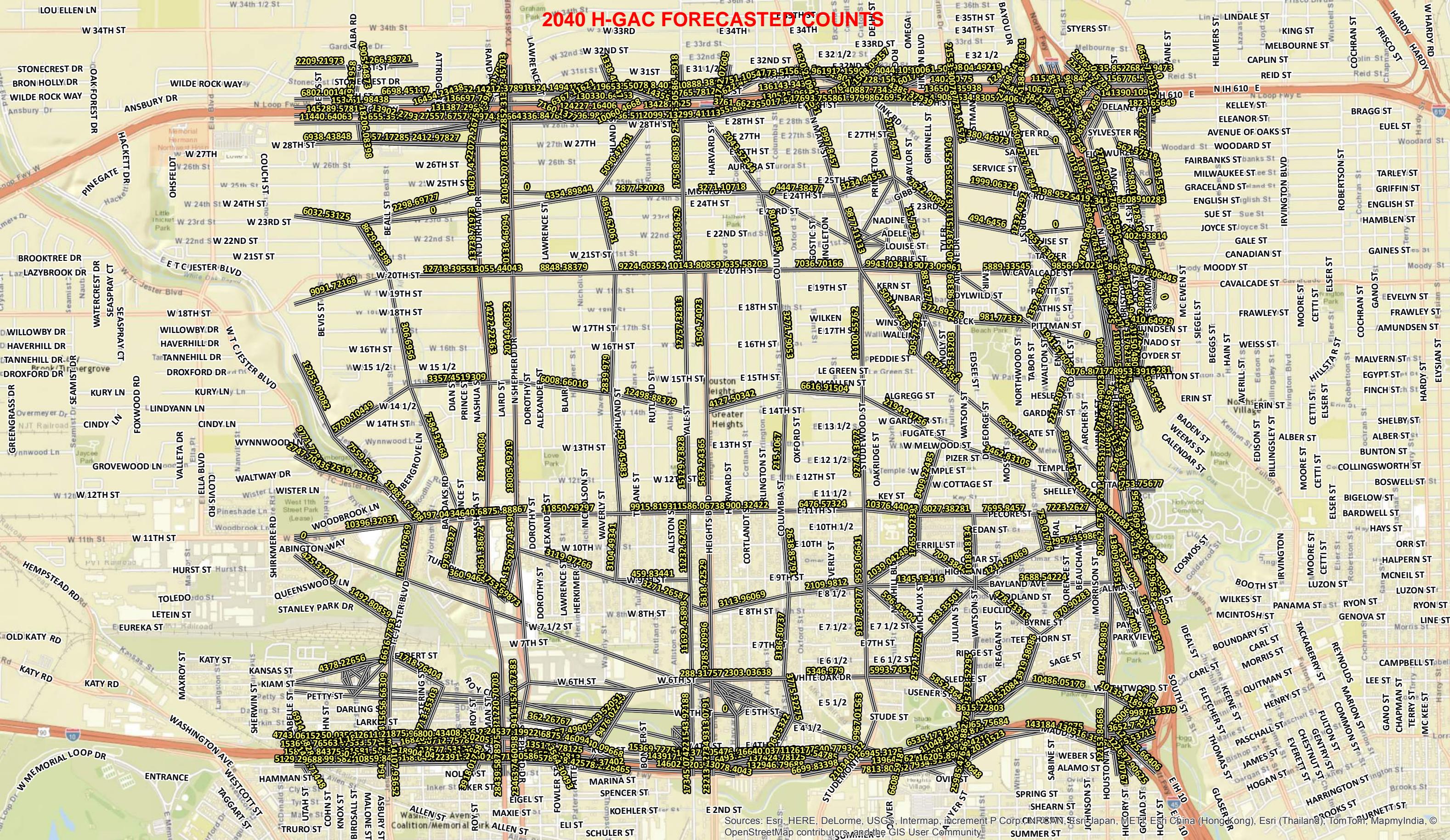
2018 H-GAC FORECASTED COUNTS



2035 H-GAC FORECASTED COUNTS



2040 H-GAC FORECASTED COUNTS



Appendix C

Capacity Analysis – Existing Conditions

TIRZ 5 TIA
8: Shepherd Dr & W 20th

Existing Conditions
Timing Plan: AM PEAK HOUR



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑		↑↑↑					
Traffic Volume (vph)	43	195	0	0	221	85	44	1000	71	0	0	0
Future Volume (vph)	43	195	0	0	221	85	44	1000	71	0	0	0
Satd. Flow (prot)	0	3500	0	0	3398	0	0	6325	0	0	0	0
Flt Permitted		0.716						0.998				
Satd. Flow (perm)	0	2534	0	0	3398	0	0	6325	0	0	0	0
Satd. Flow (RTOR)					46			24				
Lane Group Flow (vph)	0	299	0	0	391	0	0	1201	0	0	0	0
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		2			6			8				
Permitted Phases	2						8					
Total Split (s)	45.0	45.0			45.0		75.0	75.0				
Total Lost Time (s)		5.1			5.1			5.3				
Act Effct Green (s)	30.0				30.0			79.6				
Actuated g/C Ratio	0.25				0.25			0.66				
v/c Ratio	0.47				0.44			0.29				
Control Delay	27.5				35.1			9.3				
Queue Delay	0.0				0.0			0.0				
Total Delay	27.5				35.1			9.3				
LOS	C				D			A				
Approach Delay	27.5				35.1			9.3				
Approach LOS	C				D			A				
Queue Length 50th (ft)	63				118			169				
Queue Length 95th (ft)	87				136			m156				
Internal Link Dist (ft)	158				1611			4775				1423
Turn Bay Length (ft)												
Base Capacity (vph)	842				1160			4203				
Starvation Cap Reductn	0				0			0				
Spillback Cap Reductn	0				0			0				
Storage Cap Reductn	0				0			0				
Reduced v/c Ratio	0.36				0.34			0.29				

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.47

Intersection Signal Delay: 17.5

Intersection LOS: B

Intersection Capacity Utilization 82.2%

ICU Level of Service E

Analysis Period (min) 15

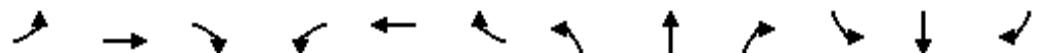
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 8: Shepherd Dr & W 20th



TIRZ 5 TIA
11: Durham Dr & W 20th St

Existing Conditions
Timing Plan: AM PEAK HOUR



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↑		↑					↑↑↑	↑↑↑	
Traffic Volume (vph)	0	131	29	91	181	0	0	0	0	113	2023	138
Future Volume (vph)	0	131	29	91	181	0	0	0	0	113	2023	138
Satd. Flow (prot)	0	1863	1583	0	1835	0	0	0	0	0	6325	0
Flt Permitted						0.721						0.997
Satd. Flow (perm)	0	1863	1583	0	1343	0	0	0	0	0	6325	0
Satd. Flow (RTOR)				21							21	
Lane Group Flow (vph)	0	177	36	0	338	0	0	0	0	0	2452	0
Turn Type	NA	Perm	Perm	NA						Perm	NA	
Protected Phases		2				6						4
Permitted Phases			2	6							4	
Total Split (s)	45.0	45.0	45.0	45.0						75.0	75.0	
Total Lost Time (s)	5.0	5.0		5.0							5.3	
Act Effct Green (s)	32.9	32.9		32.9							76.8	
Actuated g/C Ratio	0.27	0.27		0.27							0.64	
v/c Ratio	0.35	0.08		0.92							0.60	
Control Delay	35.7	16.4		57.2							14.0	
Queue Delay	0.0	0.0		0.0							0.0	
Total Delay	35.7	16.4		57.2							14.0	
LOS	D	B		E							B	
Approach Delay	32.5			57.2							14.0	
Approach LOS	C			E							B	
Queue Length 50th (ft)	110	9		102							300	
Queue Length 95th (ft)	128	28		83							393	
Internal Link Dist (ft)	1523			312			4811				1412	
Turn Bay Length (ft)		10										
Base Capacity (vph)	621	541		447							4056	
Starvation Cap Reductn	0	0		0							0	
Spillback Cap Reductn	0	0		0							0	
Storage Cap Reductn	0	0		0							0	
Reduced v/c Ratio	0.29	0.07		0.76							0.60	

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.92

Intersection Signal Delay: 20.2

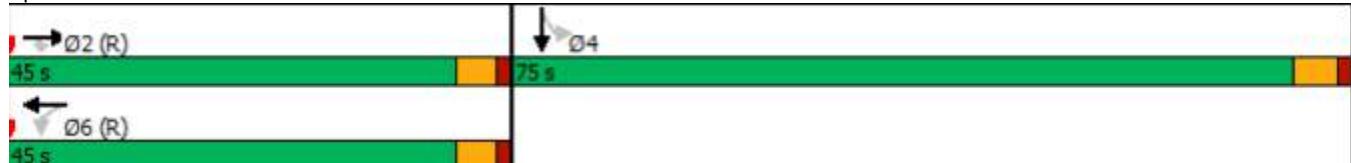
Intersection LOS: C

Intersection Capacity Utilization 96.1%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 11: Durham Dr & W 20th St



TIRZ 5 TIA
20: Shepherd Dr & 11th St

Existing Conditions
Timing Plan: AM PEAK HOUR



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑↑		↑↑↑	↑↑↑				
Traffic Volume (vph)	108	549	0	0	595	106	214	879	129	0	0	0
Future Volume (vph)	108	549	0	0	595	106	214	879	129	0	0	0
Satd. Flow (prot)	1770	3539	0	0	3451	0	0	6236	0	0	0	0
Flt Permitted	0.950								0.991			
Satd. Flow (perm)	1770	3539	0	0	3451	0	0	6236	0	0	0	0
Satd. Flow (RTOR)						17			42			
Lane Group Flow (vph)	144	590	0	0	904	0	0	1349	0	0	0	0
Turn Type	Prot	NA			NA		Perm		NA			
Protected Phases	5	2			6				8			
Permitted Phases								8				
Total Split (s)	24.0	52.0			28.0			68.0	68.0			
Total Lost Time (s)	5.2	5.2			5.2				5.0			
Act Effct Green (s)	16.2	44.2			22.8				65.6			
Actuated g/C Ratio	0.14	0.37			0.19				0.55			
v/c Ratio	0.60	0.45			1.35				0.39			
Control Delay	33.6	10.5			205.3				15.7			
Queue Delay	0.0	0.0			0.0				0.0			
Total Delay	33.6	10.5			205.3				15.7			
LOS	C	B			F				B			
Approach Delay		15.0			205.3				15.7			
Approach LOS		B			F				B			
Queue Length 50th (ft)	116	82			~480				158			
Queue Length 95th (ft)	m136	m95			#501				199			
Internal Link Dist (ft)		472			1073				2200			4775
Turn Bay Length (ft)	80											
Base Capacity (vph)	277	1380			669				3426			
Starvation Cap Reductn	0	0			0				0			
Spillback Cap Reductn	0	0			0				0			
Storage Cap Reductn	0	0			0				0			
Reduced v/c Ratio	0.52	0.43			1.35				0.39			

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.35

Intersection Signal Delay: 72.9

Intersection LOS: E

Intersection Capacity Utilization 80.4%

ICU Level of Service D

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

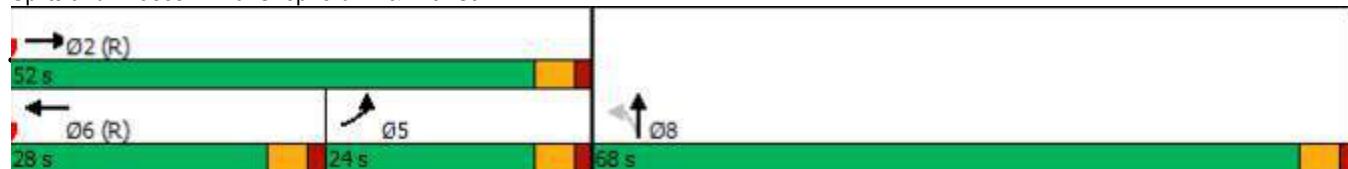
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Shepherd Dr & 11th St



TIRZ 5 TIA
22: Durham Dr & 11th St

Existing Conditions
Timing Plan: AM PEAK HOUR



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↓	↑	↑	↑↓					↑↑↑↓		
Traffic Volume (vph)	0	434	193	219	478	0	0	0	0	232	1918	160
Future Volume (vph)	0	434	193	219	478	0	0	0	0	232	1918	160
Satd. Flow (prot)	0	3370	1441	1770	3539	0	0	0	0	0	6293	0
Flt Permitted					0.950						0.995	
Satd. Flow (perm)	0	3370	1441	1770	3539	0	0	0	0	0	6293	0
Satd. Flow (RTOR)		3	68								25	
Lane Group Flow (vph)	0	532	193	277	569	0	0	0	0	0	2651	0
Turn Type	NA	Perm	Prot	NA						Perm	NA	
Protected Phases		2		1	6						4	
Permitted Phases			2								4	
Total Split (s)		31.0	31.0	24.0	55.0					65.0	65.0	
Total Lost Time (s)		5.2	5.2	5.2	5.2						5.3	
Act Effct Green (s)		25.4	25.4	19.2	49.8						59.7	
Actuated g/C Ratio		0.21	0.21	0.16	0.42						0.50	
v/c Ratio		0.74	0.54	0.98	0.39						0.84	
Control Delay		51.2	33.0	45.9	9.7						36.9	
Queue Delay		0.0	0.0	0.0	0.0						0.0	
Total Delay		51.2	33.0	45.9	9.7						36.9	
LOS	D	C	D	A							D	
Approach Delay		46.3			21.5						36.9	
Approach LOS		D			C						D	
Queue Length 50th (ft)		212	93	230	56						613	
Queue Length 95th (ft)		258	179	m196	m47						637	
Internal Link Dist (ft)		797			472				1903		4811	
Turn Bay Length (ft)			125	100								
Base Capacity (vph)		726	363	283	1468						3143	
Starvation Cap Reductn	0	0	0	0							0	
Spillback Cap Reductn	0	0	0	0							0	
Storage Cap Reductn	0	0	0	0							0	
Reduced v/c Ratio	0.73	0.53	0.98	0.39							0.84	

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.98

Intersection Signal Delay: 35.4

Intersection LOS: D

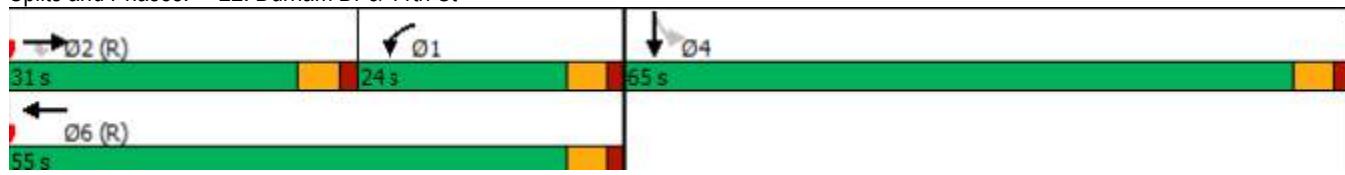
Intersection Capacity Utilization 80.4%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 22: Durham Dr & 11th St



TIRZ 5 TIA
8: Shepherd Dr & W 20th

Existing Conditions
Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑		↓↓↓					
Traffic Volume (vph)	79	332	0	0	230	151	57	2176	113	0	0	0
Future Volume (vph)	79	332	0	0	230	151	57	2176	113	0	0	0
Satd. Flow (prot)	0	3507	0	0	3330	0	0	6344	0	0	0	0
Flt Permitted		0.677						0.998				
Satd. Flow (perm)	0	2396	0	0	3330	0	0	6344	0	0	0	0
Satd. Flow (RTOR)					5			17				
Lane Group Flow (vph)	0	483	0	0	436	0	0	2487	0	0	0	0
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		2				6			8			
Permitted Phases	2							8				
Total Split (s)	45.0	45.0			45.0		75.0	75.0				
Total Lost Time (s)		5.1			5.1			5.3				
Act Effct Green (s)		32.2			32.2			77.4				
Actuated g/C Ratio		0.27			0.27			0.64				
v/c Ratio		0.75			0.49			0.61				
Control Delay		39.0			38.2			2.2				
Queue Delay		0.0			0.0			0.0				
Total Delay		39.0			38.2			2.2				
LOS		D			D			A				
Approach Delay		39.0			38.2			2.2				
Approach LOS		D			D			A				
Queue Length 50th (ft)		141			152			33				
Queue Length 95th (ft)		163			178			m68				
Internal Link Dist (ft)		158			1495			4775			1423	
Turn Bay Length (ft)												
Base Capacity (vph)		796			1110			4098				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.61			0.39			0.61				

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.75

Intersection Signal Delay: 12.0

Intersection LOS: B

Intersection Capacity Utilization 100.4%

ICU Level of Service G

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 8: Shepherd Dr & W 20th



TIRZ 5 TIA
11: Durham Dr & W 20th St

Existing Conditions
Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↑		↑					↑↑↑	↑↑↑	
Traffic Volume (vph)	0	295	34	88	197	0	0	0	0	117	1377	96
Future Volume (vph)	0	295	34	88	197	0	0	0	0	117	1377	96
Satd. Flow (prot)	0	1863	1583	0	1835	0	0	0	0	0	6318	0
Flt Permitted					0.600						0.996	
Satd. Flow (perm)	0	1863	1583	0	1118	0	0	0	0	0	6318	0
Satd. Flow (RTOR)				42							37	
Lane Group Flow (vph)	0	378	36	0	324	0	0	0	0	0	1688	0
Turn Type	NA	Perm	Perm	NA						Perm	NA	
Protected Phases		2				6						4
Permitted Phases			2	6							4	
Total Split (s)	25.0	25.0	25.0	25.0						35.0	35.0	
Total Lost Time (s)	5.0	5.0		5.0							5.3	
Act Effct Green (s)	20.0	20.0		20.0							29.7	
Actuated g/C Ratio	0.33	0.33		0.33							0.50	
v/c Ratio	0.61	0.06		0.87							0.54	
Control Delay	21.8	5.1		59.2							10.9	
Queue Delay	0.0	0.0		0.0							0.0	
Total Delay	21.8	5.1		59.2							10.9	
LOS	C	A		E							B	
Approach Delay	20.3			59.2							10.9	
Approach LOS	C			E							B	
Queue Length 50th (ft)	112	0		179							110	
Queue Length 95th (ft)	156	14		#283							139	
Internal Link Dist (ft)	1438			312			4811				1412	
Turn Bay Length (ft)		10										
Base Capacity (vph)	621	555		372							3146	
Starvation Cap Reductn	0	0		0							0	
Spillback Cap Reductn	0	0		0							0	
Storage Cap Reductn	0	0		0							0	
Reduced v/c Ratio	0.61	0.06		0.87							0.54	

Intersection Summary

Cycle Length: 60

Actuated Cycle Length: 60

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.87

Intersection Signal Delay: 19.0

Intersection LOS: B

Intersection Capacity Utilization 69.4%

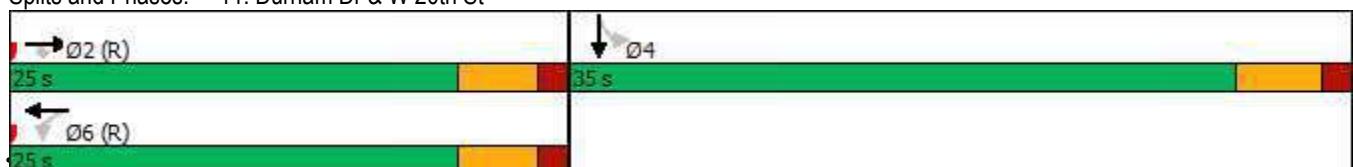
ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

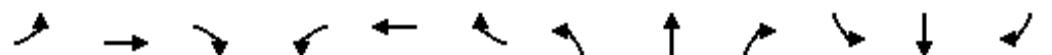
Queue shown is maximum after two cycles.

Splits and Phases: 11: Durham Dr & W 20th St



TIRZ 5 TIA
20: Shepherd Dr & 11th St

Existing Conditions
Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑↑		↓↓↓					
Traffic Volume (vph)	251	779	0	0	541	131	342	2102	120	0	0	0
Future Volume (vph)	251	779	0	0	541	131	342	2102	120	0	0	0
Satd. Flow (prot)	1770	3539	0	0	3429	0	0	6318	0	0	0	0
Flt Permitted	0.950							0.994				
Satd. Flow (perm)	1770	3539	0	0	3429	0	0	6318	0	0	0	0
Satd. Flow (RTOR)					26			13				
Lane Group Flow (vph)	264	847	0	0	812	0	0	2768	0	0	0	0
Turn Type	Prot	NA			NA		Perm	NA				
Protected Phases	5	2			6			8				
Permitted Phases							8					
Total Split (s)	24.0	58.0			34.0		62.0	62.0				
Total Lost Time (s)	5.2	5.2			5.2			5.0				
Act Effct Green (s)	18.8	52.8			28.8			57.0				
Actuated g/C Ratio	0.16	0.44			0.24			0.48				
v/c Ratio	0.95	0.54			0.96			0.92				
Control Delay	78.1	16.1			67.4			35.5				
Queue Delay	0.0	0.7			0.0			0.0				
Total Delay	78.1	16.8			67.4			35.5				
LOS	E	B			E			D				
Approach Delay		31.4			67.4			35.5				
Approach LOS		C			E			D				
Queue Length 50th (ft)	204	266			320			564				
Queue Length 95th (ft)	#375	331			#396			621				
Internal Link Dist (ft)		472			1073			2200			4775	
Turn Bay Length (ft)	80											
Base Capacity (vph)	277	1557			842			3007				
Starvation Cap Reductn	0	366			0			0				
Spillback Cap Reductn	0	0			0			0				
Storage Cap Reductn	0	0			0			0				
Reduced v/c Ratio	0.95	0.71			0.96			0.92				

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 24 (20%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.96

Intersection Signal Delay: 40.0

Intersection LOS: D

Intersection Capacity Utilization 84.3%

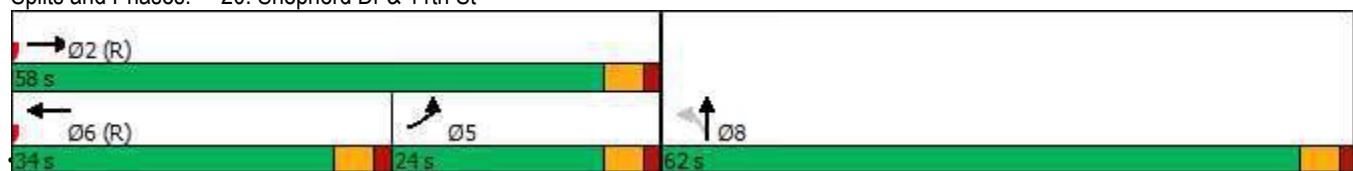
ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 20: Shepherd Dr & 11th St



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	848	185	167	641	0	0	0	0	295	1062	114
Future Volume (vph)	0	848	185	167	641	0	0	0	0	295	1062	114
Satd. Flow (prot)	0	3377	1441	1770	3539	0	0	0	0	0	6268	0
Flt Permitted					0.950						0.990	
Satd. Flow (perm)	0	3377	1441	1770	3539	0	0	0	0	0	6268	0
Satd. Flow (RTOR)		2	68								17	
Lane Group Flow (vph)	0	905	193	184	737	0	0	0	0	0	1665	0
Turn Type	NA	Perm	Prot	NA						Perm	NA	
Protected Phases		4		3	8						6	
Permitted Phases			4								6	
Total Split (s)		52.4	52.4	20.6	73.0					47.0	47.0	
Total Lost Time (s)		5.2	5.2	5.2	5.2						5.3	
Act Effct Green (s)		41.0	41.0	15.3	61.5						48.0	
Actuated g/C Ratio		0.34	0.34	0.13	0.51						0.40	
v/c Ratio		0.78	0.36	0.81	0.41						0.66	
Control Delay		40.4	19.7	67.3	28.0						26.9	
Queue Delay		0.0	0.0	0.0	0.0						0.0	
Total Delay		40.4	19.7	67.3	28.0						26.9	
LOS	D	B	E	C							C	
Approach Delay		36.7			35.9						26.9	
Approach LOS		D			D						C	
Queue Length 50th (ft)		338	75	143	195						220	
Queue Length 95th (ft)		392	125	m152	m161						391	
Internal Link Dist (ft)		797			472			1903			4811	
Turn Bay Length (ft)		125	100									
Base Capacity (vph)		1329	608	227	1999						2516	
Starvation Cap Reductn	0	0	0	0							0	
Spillback Cap Reductn	0	0	0	0							0	
Storage Cap Reductn	0	0	0	0							0	
Reduced v/c Ratio	0.68	0.32	0.81	0.37							0.66	

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 52.4 (44%), Referenced to phase 2: and 6:SBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 32.1

Intersection LOS: C

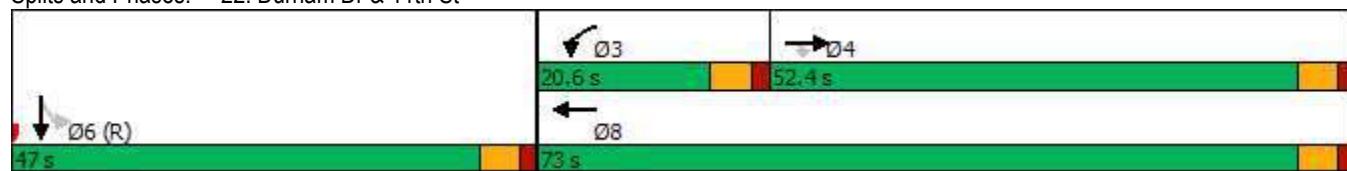
Intersection Capacity Utilization 84.3%

ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

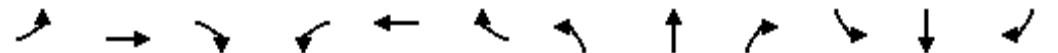
Splits and Phases: 22: Durham Dr & 11th St



Appendix D

Capacity Analysis – 2040 Projected Conditions –

Scenario 1



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑		↑↑↑	↑↑↑				
Traffic Volume (vph)	51	231	0	0	261	100	52	1182	84	0	0	0
Future Volume (vph)	51	231	0	0	261	100	52	1182	84	0	0	0
Satd. Flow (prot)	0	3500	0	0	3398	0	0	6325	0	0	0	0
Flt Permitted									0.998			
Satd. Flow (perm)	0	2293	0	0	3398	0	0	6325	0	0	0	0
Satd. Flow (RTOR)						45			24			
Lane Group Flow (vph)	0	354	0	0	461	0	0	1419	0	0	0	0
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		2				6			8			
Permitted Phases	2								8			
Total Split (s)	45.0	45.0			45.0		75.0	75.0				
Total Lost Time (s)		5.1			5.1			5.3				
Act Effct Green (s)	30.0				30.0			79.6				
Actuated g/C Ratio	0.25				0.25			0.66				
v/c Ratio	0.62				0.52			0.34				
Control Delay	29.9				37.3			8.8				
Queue Delay		0.0			0.0			0.0				
Total Delay	29.9				37.3			8.8				
LOS	C				D			A				
Approach Delay	29.9				37.3			8.8				
Approach LOS	C				D			A				
Queue Length 50th (ft)	82				146			124				
Queue Length 95th (ft)	104				164			143				
Internal Link Dist (ft)	158				1611			4775			1423	
Turn Bay Length (ft)												
Base Capacity (vph)	762				1159			4203				
Starvation Cap Reductn	0				0			0				
Spillback Cap Reductn	0				0			0				
Storage Cap Reductn	0				0			0				
Reduced v/c Ratio	0.46				0.40			0.34				

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.62

Intersection Signal Delay: 18.1

Intersection LOS: B

Intersection Capacity Utilization 88.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 8: Shepherd Dr & W 20th



TIRZ 5 TIA
11: Durham Dr & W 20th St

2040 Projected Conditions
Timing Plan: AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↑		↑					↑↑↑↑		
Traffic Volume (vph)	0	155	34	108	214	0	0	0	0	134	2392	163
Future Volume (vph)	0	155	34	108	214	0	0	0	0	134	2392	163
Satd. Flow (prot)	0	1863	1583	0	1835	0	0	0	0	0	6325	0
Flt Permitted						0.699						0.997
Satd. Flow (perm)	0	1863	1583	0	1302	0	0	0	0	0	6325	0
Satd. Flow (RTOR)				21							21	
Lane Group Flow (vph)	0	209	42	0	401	0	0	0	0	0	2900	0
Turn Type	NA	Perm	Perm	NA						Perm	NA	
Protected Phases		2				6						4
Permitted Phases			2	6								4
Total Split (s)	45.0	45.0	45.0	45.0						75.0	75.0	
Total Lost Time (s)	5.0	5.0		5.0							5.3	
Act Effct Green (s)	38.5	38.5		38.5							71.2	
Actuated g/C Ratio	0.32	0.32		0.32							0.59	
v/c Ratio	0.35	0.08		0.96							0.77	
Control Delay	32.7	17.0		58.5							20.3	
Queue Delay	0.0	0.0		0.0							0.0	
Total Delay	32.7	17.0		58.5							20.3	
LOS	C	B		E							C	
Approach Delay	30.1			58.5							20.3	
Approach LOS	C			E							C	
Queue Length 50th (ft)	121	11		118							475	
Queue Length 95th (ft)	149	32		#151							522	
Internal Link Dist (ft)	1523			312				4811			1412	
Turn Bay Length (ft)		10										
Base Capacity (vph)	621	541		434							3759	
Starvation Cap Reductn	0	0		0							0	
Spillback Cap Reductn	0	0		0							0	
Storage Cap Reductn	0	0		0							0	
Reduced v/c Ratio	0.34	0.08		0.92							0.77	

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.96

Intersection Signal Delay: 25.3

Intersection LOS: C

Intersection Capacity Utilization 96.1%

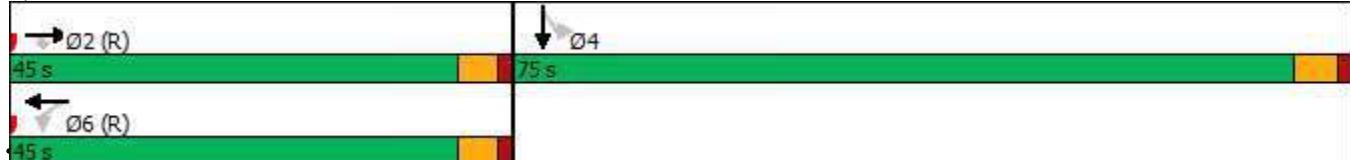
ICU Level of Service F

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 11: Durham Dr & W 20th St



TIRZ 5 TIA
20: Shepherd Dr & 11th St

2040 Projected Conditions
Timing Plan: AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑↑			↑↑↑				
Traffic Volume (vph)	128	649	0	0	703	125	253	1039	153	0	0	0
Future Volume (vph)	128	649	0	0	703	125	253	1039	153	0	0	0
Satd. Flow (prot)	1770	3539	0	0	3451	0	0	6236	0	0	0	0
Flt Permitted	0.950								0.991			
Satd. Flow (perm)	1770	3539	0	0	3451	0	0	6236	0	0	0	0
Satd. Flow (RTOR)						17			36			
Lane Group Flow (vph)	171	698	0	0	1069	0	0	1596	0	0	0	0
Turn Type	Prot	NA			NA		Perm	NA				
Protected Phases	5	2			6			8				
Permitted Phases							8					
Total Split (s)	24.0	62.0			38.0		68.0	68.0				
Total Lost Time (s)	5.2	5.2			5.2			5.0				
Act Effct Green (s)	17.1	55.1			32.8			64.7				
Actuated g/C Ratio	0.13	0.42			0.25			0.50				
v/c Ratio	0.74	0.47			1.21			0.51				
Control Delay	73.1	27.9			146.4			22.3				
Queue Delay	0.0	0.0			0.0			0.0				
Total Delay	73.1	27.9			146.4			22.3				
LOS	E	C			F			C				
Approach Delay	36.8				146.4			22.3				
Approach LOS	D				F			C				
Queue Length 50th (ft)	140	216			~574			253				
Queue Length 95th (ft)	177	267			#573			292				
Internal Link Dist (ft)		472			1073			2200				4775
Turn Bay Length (ft)	80											
Base Capacity (vph)	255	1546			883			3120				
Starvation Cap Reductn	0	0			0			0				
Spillback Cap Reductn	0	0			0			0				
Storage Cap Reductn	0	0			0			0				
Reduced v/c Ratio	0.67	0.45			1.21			0.51				

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.21

Intersection Signal Delay: 63.4

Intersection LOS: E

Intersection Capacity Utilization 88.5%

ICU Level of Service E

Analysis Period (min) 15

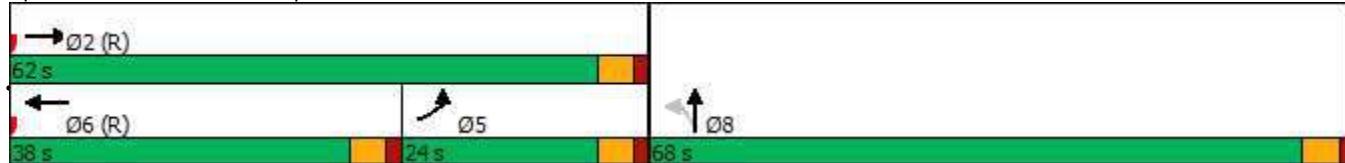
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 20: Shepherd Dr & 11th St



TIRZ 5 TIA
22: Durham Dr & 11th St

2040 Projected Conditions

Timing Plan: AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↓	↑	↑	↑↓					↑↑↑↓		
Traffic Volume (vph)	0	513	228	259	565	0	0	0	0	274	2268	189
Future Volume (vph)	0	513	228	259	565	0	0	0	0	274	2268	189
Satd. Flow (prot)	0	3370	1441	1770	3539	0	0	0	0	0	6293	0
Flt Permitted						0.950						0.995
Satd. Flow (perm)	0	3370	1441	1770	3539	0	0	0	0	0	6293	0
Satd. Flow (RTOR)		3	55									18
Lane Group Flow (vph)	0	629	228	328	673	0	0	0	0	0	3132	0
Turn Type	NA	Perm	Prot	NA						Perm	NA	
Protected Phases	2		1	6								4
Permitted Phases			2									4
Total Split (s)	52.0	52.0	28.0	80.0						70.0	70.0	
Total Lost Time (s)	5.2	5.2	5.2	5.2								5.3
Act Effct Green (s)	35.8	35.8	33.8	74.8								64.7
Actuated g/C Ratio	0.24	0.24	0.23	0.50								0.43
v/c Ratio	0.78	0.59	0.82	0.38								1.15
Control Delay	60.0	43.5	72.9	24.1								111.0
Queue Delay	0.0	0.0	0.0	0.7								0.0
Total Delay	60.0	43.5	72.9	24.8								111.0
LOS	E	D	E	C								F
Approach Delay	55.6			40.6								111.0
Approach LOS	E			D								F
Queue Length 50th (ft)	317	167	308	210								~1043
Queue Length 95th (ft)	337	249	#458	237								#1056
Internal Link Dist (ft)	797			472				1903				4811
Turn Bay Length (ft)		125	100									
Base Capacity (vph)	1053	487	399	1764								2724
Starvation Cap Reductn	0	0	0	716								0
Spillback Cap Reductn	0	0	0	0								0
Storage Cap Reductn	0	0	0	0								0
Reduced v/c Ratio	0.60	0.47	0.82	0.64								1.15

Intersection Summary

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.15

Intersection Signal Delay: 87.3

Intersection LOS: F

Intersection Capacity Utilization 88.5%

ICU Level of Service E

Analysis Period (min) 15

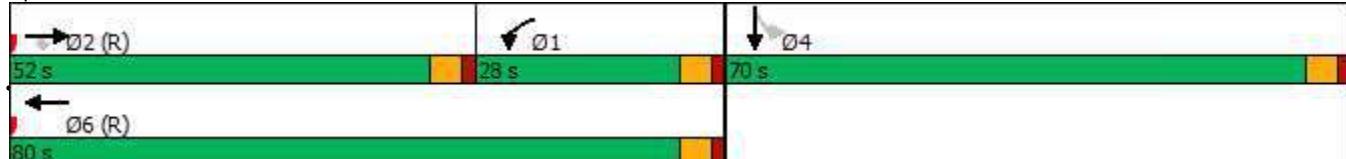
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 22: Durham Dr & 11th St



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑		↑↑↑	↑↑↑				
Traffic Volume (vph)	93	393	0	0	272	179	67	2573	134	0	0	0
Future Volume (vph)	93	393	0	0	272	179	67	2573	134	0	0	0
Satd. Flow (prot)	0	3507	0	0	3330	0	0	6344	0	0	0	0
Flt Permitted		0.645						0.998				
Satd. Flow (perm)	0	2283	0	0	3330	0	0	6344	0	0	0	0
Satd. Flow (RTOR)						2			17			
Lane Group Flow (vph)	0	570	0	0	516	0	0	2939	0	0	0	0
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		2				6			8			
Permitted Phases	2							8				
Total Split (s)	45.0	45.0			45.0		75.0	75.0				
Total Lost Time (s)		5.1			5.1			5.3				
Act Effct Green (s)		35.5			35.5			74.1				
Actuated g/C Ratio		0.30			0.30			0.62				
v/c Ratio		0.85			0.52			0.75				
Control Delay		51.9			36.7			12.2				
Queue Delay		0.0			0.0			0.0				
Total Delay		51.9			36.7			12.2				
LOS		D			D			B				
Approach Delay		51.9			36.7			12.2				
Approach LOS		D			D			B				
Queue Length 50th (ft)		216			173			597				
Queue Length 95th (ft)		255			208			m539				
Internal Link Dist (ft)		158			1495			4775			1423	
Turn Bay Length (ft)												
Base Capacity (vph)		759			1108			3925				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.75			0.47			0.75				

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 20.9

Intersection LOS: C

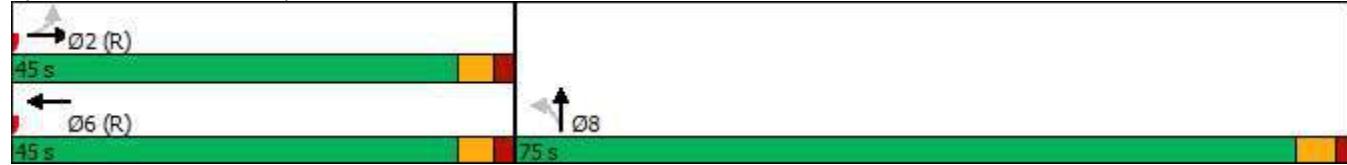
Intersection Capacity Utilization 103.5%

ICU Level of Service G

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 8: Shepherd Dr & W 20th



TIRZ 5 TIA
11: Durham Dr & W 20th St

2040 Projected Conditions
Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	349	40	104	233	0	0	0	0	138	1628	113
Future Volume (vph)	0	349	40	104	233	0	0	0	0	138	1628	113
Satd. Flow (prot)	0	1863	1583	0	1835	0	0	0	0	0	6318	0
Flt Permitted						0.494					0.996	
Satd. Flow (perm)	0	1863	1583	0	920	0	0	0	0	0	6318	0
Satd. Flow (RTOR)				28							20	
Lane Group Flow (vph)	0	447	43	0	383	0	0	0	0	0	1993	0
Turn Type	NA	Perm	Perm	NA						Perm	NA	
Protected Phases	2			6							4	
Permitted Phases		2	6								4	
Total Split (s)	50.0	50.0	50.0	50.0						40.0	40.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0							5.3	
Act Effct Green (s)	34.8	34.8	34.8	34.8							44.9	
Actuated g/C Ratio	0.39	0.39	0.39	0.39							0.50	
v/c Ratio	0.62	0.07	1.08								0.63	
Control Delay	25.2	7.2	97.4								18.9	
Queue Delay	0.0	0.0	0.0								0.0	
Total Delay	25.2	7.2	97.4								18.9	
LOS	C	A	F								B	
Approach Delay	23.6		97.4								18.9	
Approach LOS	C		F								B	
Queue Length 50th (ft)	197	5	~239								232	
Queue Length 95th (ft)	196	21	#344								334	
Internal Link Dist (ft)	1438		312				4811				1412	
Turn Bay Length (ft)		10										
Base Capacity (vph)	931	805	460								3162	
Starvation Cap Reductn	0	0	0								0	
Spillback Cap Reductn	0	0	0								0	
Storage Cap Reductn	0	0	0								0	
Reduced v/c Ratio	0.48	0.05	0.83								0.63	

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.08

Intersection Signal Delay: 30.2

Intersection LOS: C

Intersection Capacity Utilization 76.7%

ICU Level of Service D

Analysis Period (min) 15

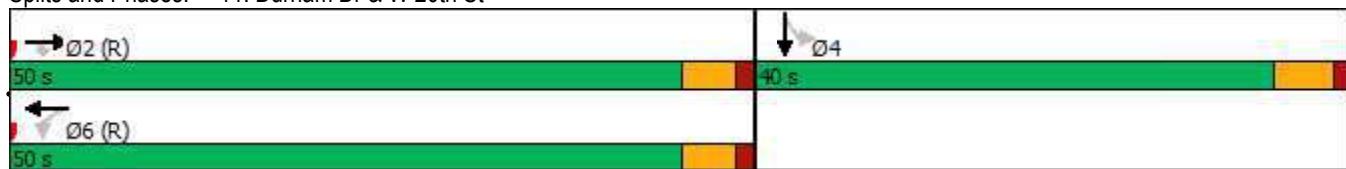
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 11: Durham Dr & W 20th St



TIRZ 5 TIA
20: Shepherd Dr & 11th St

2040 Projected Conditions

Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	297	921	0	0	640	155	404	2485	142	0	0	0
Future Volume (vph)	297	921	0	0	640	155	404	2485	142	0	0	0
Satd. Flow (prot)	1770	3539	0	0	3429	0	0	6318	0	0	0	0
Flt Permitted	0.950								0.994			
Satd. Flow (perm)	1770	3539	0	0	3429	0	0	6318	0	0	0	0
Satd. Flow (RTOR)						19			13			
Lane Group Flow (vph)	313	1001	0	0	961	0	0	3272	0	0	0	0
Turn Type	Prot	NA			NA		Perm		NA			
Protected Phases	5	2			6				8			
Permitted Phases									8			
Total Split (s)	24.0	58.0			34.0			62.0	62.0			
Total Lost Time (s)	5.2	5.2			5.2				5.0			
Act Effct Green (s)	18.8	52.8			28.8				57.0			
Actuated g/C Ratio	0.16	0.44			0.24				0.48			
v/c Ratio	1.13	0.64			1.15				1.09			
Control Delay	139.6	28.6			121.0				77.0			
Queue Delay	0.0	1.3			0.0				0.0			
Total Delay	139.6	29.9			121.0				77.0			
LOS	F	C			F				E			
Approach Delay		56.0			121.0				77.0			
Approach LOS		E			F				E			
Queue Length 50th (ft)	~281	312			~455				~828			
Queue Length 95th (ft)	#462	384			#525				#893			
Internal Link Dist (ft)		472			1073				2200			4775
Turn Bay Length (ft)	80											
Base Capacity (vph)	277	1557			837				3007			
Starvation Cap Reductn	0	332			0				0			
Spillback Cap Reductn	0	0			0				0			
Storage Cap Reductn	0	0			0				0			
Reduced v/c Ratio	1.13	0.82			1.15				1.09			

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.15

Intersection Signal Delay: 79.7

Intersection LOS: E

Intersection Capacity Utilization 96.5%

ICU Level of Service F

Analysis Period (min) 15

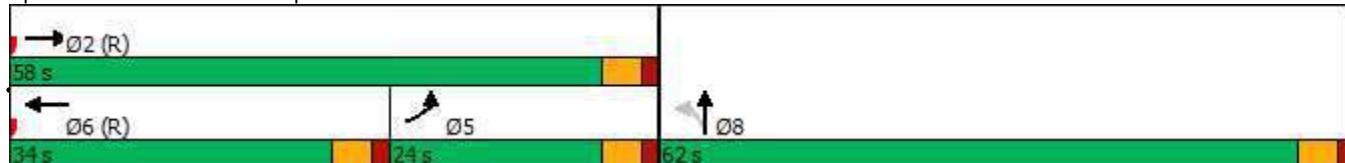
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 20: Shepherd Dr & 11th St



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑	↑	↑	↑↑					↑↑↑↑		
Traffic Volume (vph)	0	1003	219	197	758	0	0	0	0	349	1256	135
Future Volume (vph)	0	1003	219	197	758	0	0	0	0	349	1256	135
Satd. Flow (prot)	0	3377	1441	1770	3539	0	0	0	0	0	6268	0
Flt Permitted					0.950						0.990	
Satd. Flow (perm)	0	3377	1441	1770	3539	0	0	0	0	0	6268	0
Satd. Flow (RTOR)		2	65								16	
Lane Group Flow (vph)	0	1071	229	216	871	0	0	0	0	0	1969	0
Turn Type	NA	Perm	Prot	NA						Perm	NA	
Protected Phases	2		1	6							4	
Permitted Phases		2									4	
Total Split (s)		53.2	53.2	24.8	78.0					47.0	47.0	
Total Lost Time (s)		5.2	5.2	5.2	5.2						5.3	
Act Effct Green (s)		46.7	46.7	18.4	70.3						44.2	
Actuated g/C Ratio		0.37	0.37	0.15	0.56						0.35	
v/c Ratio		0.85	0.40	0.83	0.44						0.88	
Control Delay		43.3	22.2	77.4	16.4						43.9	
Queue Delay		0.0	0.0	0.0	0.6						0.0	
Total Delay		43.3	22.2	77.4	17.1						43.9	
LOS	D	C	E	B							D	
Approach Delay		39.6			29.1						43.9	
Approach LOS		D			C						D	
Queue Length 50th (ft)		424	102	170	195						444	
Queue Length 95th (ft)		519	167	#290	230						489	
Internal Link Dist (ft)		797			472			1903			4811	
Turn Bay Length (ft)		125	100									
Base Capacity (vph)		1298	593	277	2061						2228	
Starvation Cap Reductn	0	0	0	752							0	
Spillback Cap Reductn	0	0	0	0							0	
Storage Cap Reductn	0	0	0	0							0	
Reduced v/c Ratio	0.83	0.39	0.78	0.67							0.88	

Intersection Summary

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 38.9

Intersection LOS: D

Intersection Capacity Utilization 96.5%

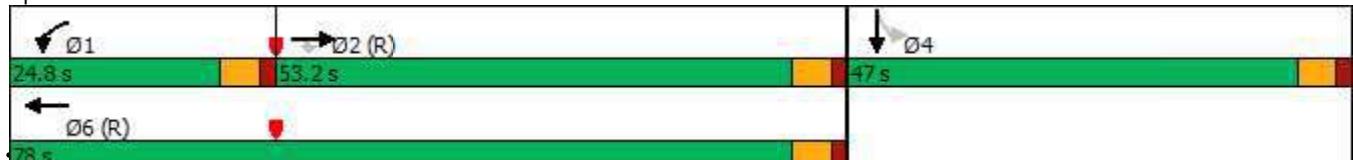
ICU Level of Service F

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 22: Durham Dr & 11th St



Appendix E

Capacity Analysis – 2040 Projected Conditions –

Scenario 2

TIRZ 5 TIA
8: Shepherd Dr & W 20th

2040 Projected Conditions - Scenario 2

Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑			↑↑↑				
Traffic Volume (vph)	51	231	0	0	261	100	52	1182	84	0	0	0
Future Volume (vph)	51	231	0	0	261	100	52	1182	84	0	0	0
Satd. Flow (prot)	0	3500	0	0	3398	0	0	5019	0	0	0	0
Flt Permitted		0.648						0.998				
Satd. Flow (perm)	0	2293	0	0	3398	0	0	5019	0	0	0	0
Satd. Flow (RTOR)					45			18				
Lane Group Flow (vph)	0	354	0	0	461	0	0	1419	0	0	0	0
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		2				6			8			
Permitted Phases	2							8				
Total Split (s)	45.0	45.0			45.0		75.0	75.0				
Total Lost Time (s)		5.1			5.1			5.3				
Act Effct Green (s)	30.0				30.0			79.6				
Actuated g/C Ratio	0.25				0.25			0.66				
v/c Ratio	0.62				0.52			0.43				
Control Delay	45.4				37.3			9.8				
Queue Delay	0.0				0.0			0.0				
Total Delay	45.4				37.3			9.8				
LOS	D				D			A				
Approach Delay	45.4				37.3			9.8				
Approach LOS	D				D			A				
Queue Length 50th (ft)	128				146			171				
Queue Length 95th (ft)	164				164			199				
Internal Link Dist (ft)	158				1611			4775			1423	
Turn Bay Length (ft)												
Base Capacity (vph)	762				1159			3335				
Starvation Cap Reductn	0				0			0				
Spillback Cap Reductn	0				0			0				
Storage Cap Reductn	0				0			0				
Reduced v/c Ratio	0.46				0.40			0.43				

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.62

Intersection Signal Delay: 21.1

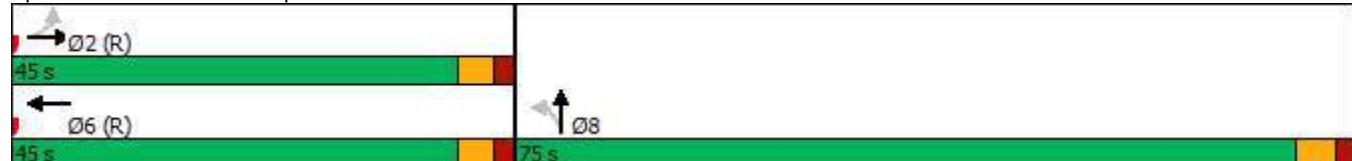
Intersection LOS: C

Intersection Capacity Utilization 88.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 8: Shepherd Dr & W 20th



TIRZ 5 TIA
11: Durham Dr & W 20th St

2040 Projected Conditions - Scenario 2

Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↑		↑					↑↑↑	↑↑↑	
Traffic Volume (vph)	0	155	34	108	214	0	0	0	0	134	2392	163
Future Volume (vph)	0	155	34	108	214	0	0	0	0	134	2392	163
Satd. Flow (prot)	0	1863	1583	0	1835	0	0	0	0	0	5019	0
Flt Permitted						0.687					0.997	
Satd. Flow (perm)	0	1863	1583	0	1280	0	0	0	0	0	5019	0
Satd. Flow (RTOR)				18							14	
Lane Group Flow (vph)	0	209	42	0	401	0	0	0	0	0	2900	0
Turn Type	NA	Perm	Perm	NA						Perm	NA	
Protected Phases	2				6						4	
Permitted Phases		2	6								4	
Total Split (s)	53.0	53.0	53.0	53.0						87.0	87.0	
Total Lost Time (s)	5.0	5.0	5.0								5.3	
Act Effct Green (s)	45.5	45.5	45.5								84.2	
Actuated g/C Ratio	0.32	0.32	0.32								0.60	
v/c Ratio	0.35	0.08	0.96								0.96	
Control Delay	37.1	20.9	82.2								36.2	
Queue Delay	0.0	0.0	0.0								0.0	
Total Delay	37.1	20.9	82.2								36.2	
LOS	D	C	F								D	
Approach Delay	34.4		82.2								36.2	
Approach LOS	C		F								D	
Queue Length 50th (ft)	141	15	349								910	
Queue Length 95th (ft)	168	37	396								#1069	
Internal Link Dist (ft)	1523		312				4811				1412	
Turn Bay Length (ft)		10										
Base Capacity (vph)	638	554	438								3022	
Starvation Cap Reductn	0	0	0								0	
Spillback Cap Reductn	0	0	0								0	
Storage Cap Reductn	0	0	0								0	
Reduced v/c Ratio	0.33	0.08	0.92								0.96	

Intersection Summary

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.96

Intersection Signal Delay: 41.3

Intersection LOS: D

Intersection Capacity Utilization 107.0%

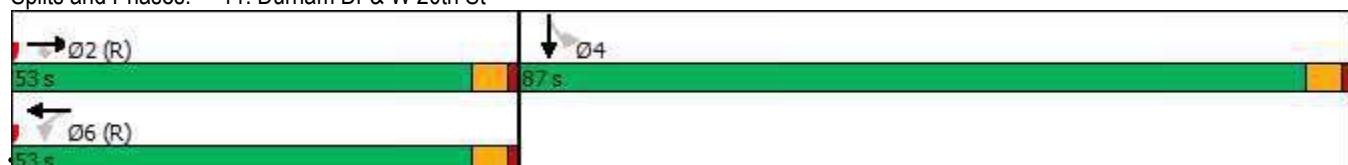
ICU Level of Service G

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 11: Durham Dr & W 20th St



TIRZ 5 TIA
20: Shepherd Dr & 11th St

2040 Projected Conditions - Scenario 2

Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑↑		↑↑	↑↑				
Traffic Volume (vph)	128	649	0	0	703	125	253	1039	153	0	0	0
Future Volume (vph)	128	649	0	0	703	125	253	1039	153	0	0	0
Satd. Flow (prot)	1770	3539	0	0	3451	0	0	4949	0	0	0	0
Flt Permitted	0.950								0.991			
Satd. Flow (perm)	1770	3539	0	0	3451	0	0	4949	0	0	0	0
Satd. Flow (RTOR)						17			26			
Lane Group Flow (vph)	171	698	0	0	1069	0	0	1596	0	0	0	0
Turn Type	Prot	NA			NA		Perm	NA				
Protected Phases	5	2			6				8			
Permitted Phases								8				
Total Split (s)	24.0	62.0			38.0		68.0	68.0				
Total Lost Time (s)	5.2	5.2			5.2			5.0				
Act Effct Green (s)	17.1	55.1			32.8			64.7				
Actuated g/C Ratio	0.13	0.42			0.25			0.50				
v/c Ratio	0.74	0.47			1.21			0.64				
Control Delay	73.1	27.9			146.4			25.4				
Queue Delay	0.0	0.0			0.0			0.0				
Total Delay	73.1	27.9			146.4			25.4				
LOS	E	C			F			C				
Approach Delay	36.8				146.4			25.4				
Approach LOS	D				F			C				
Queue Length 50th (ft)	140	216			~574			353				
Queue Length 95th (ft)	177	267			#573			412				
Internal Link Dist (ft)		472			1073			2200			4775	
Turn Bay Length (ft)	80											
Base Capacity (vph)	255	1546			883			2475				
Starvation Cap Reductn	0	0			0			0				
Spillback Cap Reductn	0	0			0			0				
Storage Cap Reductn	0	0			0			0				
Reduced v/c Ratio	0.67	0.45			1.21			0.64				

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.21

Intersection Signal Delay: 64.8

Intersection LOS: E

Intersection Capacity Utilization 101.9%

ICU Level of Service G

Analysis Period (min) 15

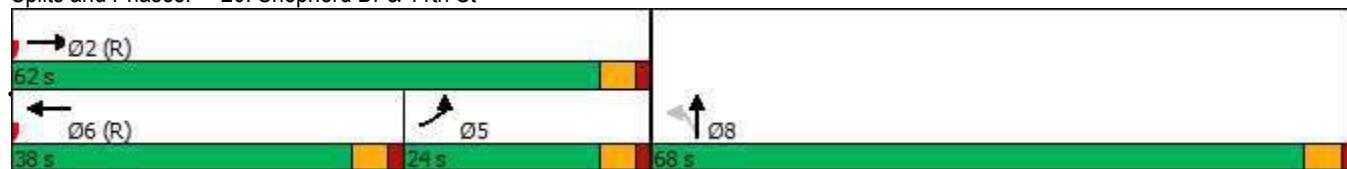
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 20: Shepherd Dr & 11th St



TIRZ 5 TIA
22: Durham Dr & 11th St

2040 Projected Conditions - Scenario 2

Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↓	↑	↑	↑↓					↑↓↑↓		
Traffic Volume (vph)	0	513	228	259	565	0	0	0	0	274	2268	189
Future Volume (vph)	0	513	228	259	565	0	0	0	0	274	2268	189
Satd. Flow (prot)	0	3370	1441	1770	3539	0	0	0	0	0	4994	0
Flt Permitted					0.950						0.995	
Satd. Flow (perm)	0	3370	1441	1770	3539	0	0	0	0	0	4994	0
Satd. Flow (RTOR)		3	55								13	
Lane Group Flow (vph)	0	629	228	328	673	0	0	0	0	0	3132	0
Turn Type	NA	Perm	Prot	NA						Perm	NA	
Protected Phases		2		1	6						4	
Permitted Phases			2								4	
Total Split (s)		51.0	51.0	26.0	77.0					73.0	73.0	
Total Lost Time (s)		5.2	5.2	5.2	5.2						5.3	
Act Effct Green (s)		35.7	35.7	30.9	71.8						67.7	
Actuated g/C Ratio		0.24	0.24	0.21	0.48						0.45	
v/c Ratio		0.78	0.59	0.90	0.40						1.39	
Control Delay		60.0	43.6	85.4	26.1						209.5	
Queue Delay		0.0	0.0	0.0	0.8						0.0	
Total Delay		60.0	43.6	85.4	26.8						209.5	
LOS	E	D	F	C							F	
Approach Delay		55.7			46.0						209.5	
Approach LOS		E			D						F	
Queue Length 50th (ft)		317	167	316	219						~1493	
Queue Length 95th (ft)		337	249	#493	247						#1509	
Internal Link Dist (ft)		797			472			1903			4811	
Turn Bay Length (ft)			125	100								
Base Capacity (vph)		1031	478	364	1694						2261	
Starvation Cap Reductn	0	0	0	660							0	
Spillback Cap Reductn	0	0	0	0							0	
Storage Cap Reductn	0	0	0	0							0	
Reduced v/c Ratio	0.61	0.48	0.90	0.65							1.39	

Intersection Summary

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.39

Intersection Signal Delay: 150.3

Intersection LOS: F

Intersection Capacity Utilization 101.9%

ICU Level of Service G

Analysis Period (min) 15

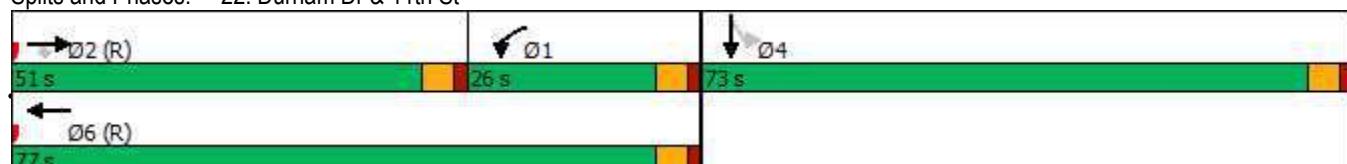
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 22: Durham Dr & 11th St



TIRZ 5 TIA
8: Shepherd Dr & W 20th

2040 Projected Conditions - Scenario 2

Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑		↑↑↑	↑↑				
Traffic Volume (vph)	93	393	0	0	272	179	67	2573	134	0	0	0
Future Volume (vph)	93	393	0	0	272	179	67	2573	134	0	0	0
Satd. Flow (prot)	0	3507	0	0	3330	0	0	5035	0	0	0	0
Flt Permitted		0.634						0.998				
Satd. Flow (perm)	0	2244	0	0	3330	0	0	5035	0	0	0	0
Satd. Flow (RTOR)						2		13				
Lane Group Flow (vph)	0	570	0	0	516	0	0	2939	0	0	0	0
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		2				6			8			
Permitted Phases	2							8				
Total Split (s)	45.0	45.0			45.0		85.0	85.0				
Total Lost Time (s)		5.1			5.1			5.3				
Act Effct Green (s)		36.8			36.8			82.8				
Actuated g/C Ratio		0.28			0.28			0.64				
v/c Ratio		0.90			0.55			0.92				
Control Delay		62.6			41.3			26.8				
Queue Delay		0.0			0.0			0.0				
Total Delay		62.6			41.3			26.8				
LOS	E		D		C							
Approach Delay		62.6			41.3			26.8				
Approach LOS	E		D		C							
Queue Length 50th (ft)		236			188			779				
Queue Length 95th (ft)		287			233			880				
Internal Link Dist (ft)		158			1495			4775			1423	
Turn Bay Length (ft)												
Base Capacity (vph)		688			1023			3210				
Starvation Cap Reductn	0		0		0			0				
Spillback Cap Reductn	0		0		0			0				
Storage Cap Reductn	0		0		0			0				
Reduced v/c Ratio		0.83			0.50			0.92				

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.92

Intersection Signal Delay: 33.7

Intersection LOS: C

Intersection Capacity Utilization 117.0%

ICU Level of Service H

Analysis Period (min) 15

Splits and Phases: 8: Shepherd Dr & W 20th





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↑		↑					↑↑↑	↑↑↑	
Traffic Volume (vph)	0	349	40	104	233	0	0	0	0	138	1628	113
Future Volume (vph)	0	349	40	104	233	0	0	0	0	138	1628	113
Satd. Flow (prot)	0	1863	1583	0	1835	0	0	0	0	0	5014	0
Flt Permitted						0.507						0.996
Satd. Flow (perm)	0	1863	1583	0	944	0	0	0	0	0	5014	0
Satd. Flow (RTOR)				28								16
Lane Group Flow (vph)	0	447	43	0	383	0	0	0	0	0	1993	0
Turn Type	NA	Perm	Perm	NA						Perm	NA	
Protected Phases		2				6						4
Permitted Phases			2	6								4
Total Split (s)	46.0	46.0	46.0	46.0	46.0					44.0	44.0	
Total Lost Time (s)	5.0	5.0		5.0								5.3
Act Effct Green (s)	35.5	35.5		35.5								44.2
Actuated g/C Ratio	0.39	0.39		0.39								0.49
v/c Ratio	0.61	0.07		1.03								0.81
Control Delay	24.8	7.7		81.8								23.9
Queue Delay	0.0	0.0		0.0								0.0
Total Delay	24.8	7.7		81.8								23.9
LOS	C	A		F								C
Approach Delay	23.3			81.8								23.9
Approach LOS	C			F								C
Queue Length 50th (ft)	189	5		208								350
Queue Length 95th (ft)	215	23		#361								#471
Internal Link Dist (ft)	1438			312			4811					1412
Turn Bay Length (ft)		10										
Base Capacity (vph)	848	736		430								2470
Starvation Cap Reductn	0	0		0								0
Spillback Cap Reductn	0	0		0								0
Storage Cap Reductn	0	0		0								0
Reduced v/c Ratio	0.53	0.06		0.89								0.81

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.03

Intersection Signal Delay: 31.5

Intersection LOS: C

Intersection Capacity Utilization 85.9%

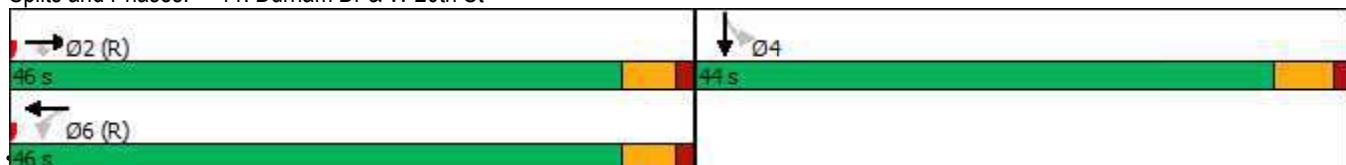
ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 11: Durham Dr & W 20th St



TIRZ 5 TIA
20: Shepherd Dr & 11th St

2040 Projected Conditions - Scenario 2

Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑↑		↑↑	↑↑				
Traffic Volume (vph)	297	921	0	0	640	155	404	2485	142	0	0	0
Future Volume (vph)	297	921	0	0	640	155	404	2485	142	0	0	0
Satd. Flow (prot)	1770	3539	0	0	3429	0	0	5014	0	0	0	0
Flt Permitted	0.950								0.994			
Satd. Flow (perm)	1770	3539	0	0	3429	0	0	5014	0	0	0	0
Satd. Flow (RTOR)						14			8			
Lane Group Flow (vph)	313	1001	0	0	961	0	0	3272	0	0	0	0
Turn Type	Prot	NA			NA		Perm	NA				
Protected Phases	5	2			6			8				
Permitted Phases								8				
Total Split (s)	27.0	69.0			42.0			81.0	81.0			
Total Lost Time (s)	5.2	5.2			5.2			5.0				
Act Effct Green (s)	21.8	63.8			36.8			76.0				
Actuated g/C Ratio	0.15	0.43			0.25			0.51				
v/c Ratio	1.22	0.67			1.13			1.29				
Control Delay	179.7	37.2			122.4			164.5				
Queue Delay	0.0	6.3			0.0			0.0				
Total Delay	179.7	43.5			122.4			164.5				
LOS	F	D			F			F				
Approach Delay		76.0			122.4			164.5				
Approach LOS		E			F			F				
Queue Length 50th (ft)	~374	409			~566			~1491				
Queue Length 95th (ft)	#571	486			#627			#1557				
Internal Link Dist (ft)		472			1073			2200			4775	
Turn Bay Length (ft)	80											
Base Capacity (vph)	257	1505			851			2544				
Starvation Cap Reductn	0	448			0			0				
Spillback Cap Reductn	0	0			0			0				
Storage Cap Reductn	0	0			0			0				
Reduced v/c Ratio	1.22	0.95			1.13			1.29				

Intersection Summary

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.29

Intersection Signal Delay: 136.3

Intersection LOS: F

Intersection Capacity Utilization 111.3%

ICU Level of Service H

Analysis Period (min) 15

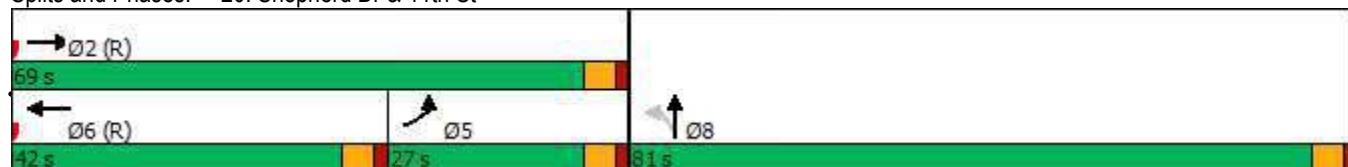
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 20: Shepherd Dr & 11th St





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↓	↑	↑	↑↓					↑↓↑↓		
Traffic Volume (vph)	0	1003	219	197	758	0	0	0	0	349	1256	135
Future Volume (vph)	0	1003	219	197	758	0	0	0	0	349	1256	135
Satd. Flow (prot)	0	3377	1441	1770	3539	0	0	0	0	0	4974	0
Flt Permitted						0.950					0.990	
Satd. Flow (perm)	0	3377	1441	1770	3539	0	0	0	0	0	4974	0
Satd. Flow (RTOR)		2	56								12	
Lane Group Flow (vph)	0	1071	229	216	871	0	0	0	0	0	1969	0
Turn Type	NA	Perm	Prot	NA						Perm	NA	
Protected Phases	2		1	6							4	
Permitted Phases			2								4	
Total Split (s)		56.0	56.0	25.0	81.0					64.0	64.0	
Total Lost Time (s)		5.2	5.2	5.2	5.2						5.3	
Act Effct Green (s)		50.2	50.2	19.4	74.7						59.8	
Actuated g/C Ratio		0.35	0.35	0.13	0.52						0.41	
v/c Ratio		0.92	0.43	0.92	0.48						0.96	
Control Delay		58.0	29.7	101.9	23.5						53.3	
Queue Delay		0.0	0.0	0.0	1.2						0.0	
Total Delay		58.0	29.7	101.9	24.7						53.3	
LOS	E	C	F	C						D		
Approach Delay		53.0			40.1					53.3		
Approach LOS		D			D					D		
Queue Length 50th (ft)		526	135	204	266					661		
Queue Length 95th (ft)		#659	208	#355	307					#762		
Internal Link Dist (ft)		797			472			1903			4811	
Turn Bay Length (ft)		125	100									
Base Capacity (vph)		1184	541	241	1850					2057		
Starvation Cap Reductn	0	0	0	704						0		
Spillback Cap Reductn	0	0	0	0						0		
Storage Cap Reductn	0	0	0	0						0		
Reduced v/c Ratio	0.90	0.42	0.90	0.76						0.96		

Intersection Summary

Cycle Length: 145

Actuated Cycle Length: 145

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.96

Intersection Signal Delay: 49.9

Intersection LOS: D

Intersection Capacity Utilization 111.3%

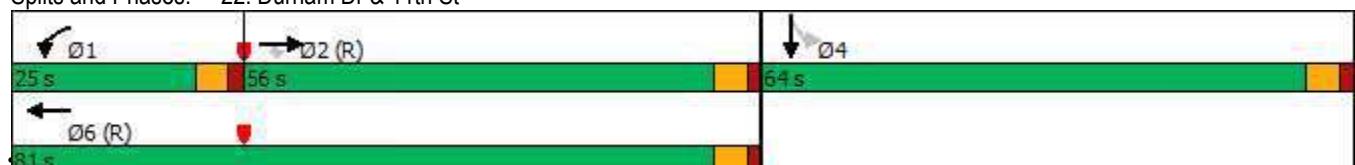
ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 22: Durham Dr & 11th St



Appendix F

Capacity Analysis – 2040 Projected Conditions –

Scenario 3



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑		↑↑↑	↑↑				
Traffic Volume (vph)	51	231	0	0	261	100	52	1182	84	0	0	0
Future Volume (vph)	51	231	0	0	261	100	52	1182	84	0	0	0
Satd. Flow (prot)	0	3500	0	0	3398	0	0	5019	0	0	0	0
Flt Permitted		0.627							0.998			
Satd. Flow (perm)	0	2219	0	0	3398	0	0	5019	0	0	0	0
Satd. Flow (RTOR)						43			16			
Lane Group Flow (vph)	0	354	0	0	461	0	0	1419	0	0	0	0
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		2				6			8			
Permitted Phases	2								8			
Total Split (s)	50.0	50.0			50.0		80.0	80.0				
Total Lost Time (s)		5.1			5.1			5.3				
Act Effct Green (s)		30.2			30.2			89.4				
Actuated g/C Ratio		0.23			0.23			0.69				
v/c Ratio		0.69			0.56			0.41				
Control Delay		37.5			42.7			9.2				
Queue Delay		0.0			0.0			0.0				
Total Delay		37.5			42.7			9.2				
LOS		D			D			A				
Approach Delay		37.5			42.7			9.2				
Approach LOS		D			D			A				
Queue Length 50th (ft)		164			165			171				
Queue Length 95th (ft)		m188			180			203				
Internal Link Dist (ft)		158			1611			4775			1423	
Turn Bay Length (ft)												
Base Capacity (vph)		766			1201			3457				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.46			0.38			0.41				

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.69

Intersection Signal Delay: 20.6

Intersection LOS: C

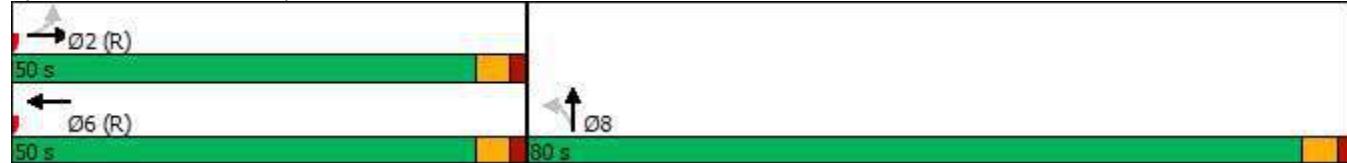
Intersection Capacity Utilization 88.8%

ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 8: Shepherd Dr & W 20th



TIRZ 5 TIA
11: Durham Dr & W 20th St

2040 Projected Conditions - Scenario 3

Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↑	↑	↑					↑↑↑	↑↑↑	
Traffic Volume (vph)	0	155	34	108	214	0	0	0	0	134	2392	163
Future Volume (vph)	0	155	34	108	214	0	0	0	0	134	2392	163
Satd. Flow (prot)	0	1863	1583	1770	1863	0	0	0	0	0	5019	0
Flt Permitted					0.446						0.997	
Satd. Flow (perm)	0	1863	1583	831	1863	0	0	0	0	0	5019	0
Satd. Flow (RTOR)				61							13	
Lane Group Flow (vph)	0	209	42	123	278	0	0	0	0	0	2900	0
Turn Type	NA	Perm	pm+pt	NA						Perm	NA	
Protected Phases	2			1	6						4	
Permitted Phases			2	6							4	
Total Split (s)	45.4	45.4	9.6	55.0						75.0	75.0	
Total Lost Time (s)	5.0	5.0	4.5	5.0							5.3	
Act Effct Green (s)	25.3	25.3	40.5	40.0							79.7	
Actuated g/C Ratio	0.19	0.19	0.31	0.31							0.61	
v/c Ratio	0.58	0.12	0.37	0.49							0.94	
Control Delay	54.5	5.6	52.4	52.9							30.6	
Queue Delay	0.0	0.0	0.0	0.0							0.0	
Total Delay	54.5	5.6	52.4	52.9							30.6	
LOS	D	A	D	D							C	
Approach Delay	46.3			52.7							30.6	
Approach LOS	D			D							C	
Queue Length 50th (ft)	162	0	68	163							771	
Queue Length 95th (ft)	192	12	122	215							857	
Internal Link Dist (ft)	1523			312			4811				1412	
Turn Bay Length (ft)		10	125									
Base Capacity (vph)	578	533	332	716							3082	
Starvation Cap Reductn	0	0	0	0							0	
Spillback Cap Reductn	0	0	0	0							0	
Storage Cap Reductn	0	0	0	0							0	
Reduced v/c Ratio	0.36	0.08	0.37	0.39							0.94	

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 110.8 (85%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.94

Intersection Signal Delay: 34.2

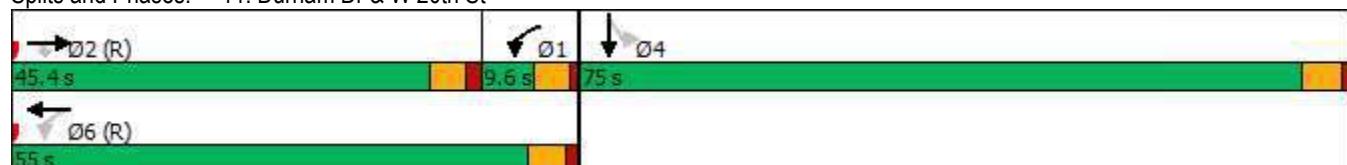
Intersection LOS: C

Intersection Capacity Utilization 94.5%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 11: Durham Dr & W 20th St



TIRZ 5 TIA
20: Shepherd Dr & 11th St

2040 Projected Conditions - Scenario 3

Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑↑	↑	↑	↑↑↑	↑			
Traffic Volume (vph)	128	649	0	0	703	125	253	1039	153	0	0	0
Future Volume (vph)	128	649	0	0	703	125	253	1039	153	0	0	0
Satd. Flow (prot)	1770	3539	0	0	3539	1583	1770	5085	1583	0	0	0
Flt Permitted	0.092								0.950			
Satd. Flow (perm)	171	3539	0	0	3539	1583	1770	5085	1583	0	0	0
Satd. Flow (RTOR)							71				171	
Lane Group Flow (vph)	171	698	0	0	890	179	278	1129	189	0	0	0
Turn Type	pm+pt	NA			NA	Perm	Perm	NA	Perm			
Protected Phases	5	2			6				8			
Permitted Phases	2					6		8		8		
Total Split (s)	24.0	77.0			53.0	53.0	68.0	68.0	68.0			
Total Lost Time (s)	5.2	5.2			5.2	5.2	5.0	5.0	5.0			
Act Effct Green (s)	63.9	63.9			44.4	44.4	70.9	70.9	70.9			
Actuated g/C Ratio	0.44	0.44			0.31	0.31	0.49	0.49	0.49			
v/c Ratio	0.74	0.45			0.82	0.34	0.32	0.45	0.22			
Control Delay	41.3	30.8			53.6	24.2	24.9	25.8	5.0			
Queue Delay	0.0	0.4			0.0	0.0	0.0	0.0	0.0			
Total Delay	41.3	31.2			53.6	24.2	24.9	25.8	5.0			
LOS	D	C			D	C	C	C	A			
Approach Delay	33.2				48.7				23.2			
Approach LOS	C				D				C			
Queue Length 50th (ft)	136	333			408	76	157	252	9			
Queue Length 95th (ft)	145	366			401	92	250	325	41			
Internal Link Dist (ft)		472			1073			2200			4775	
Turn Bay Length (ft)	200					100	200		200			
Base Capacity (vph)	282	1752			1166	569	865	2487	861			
Starvation Cap Reductn	0	558			0	0	0	0	0			
Spillback Cap Reductn	0	0			0	0	0	0	0			
Storage Cap Reductn	0	0			0	0	0	0	0			
Reduced v/c Ratio	0.61	0.58			0.76	0.31	0.32	0.45	0.22			

Intersection Summary

Cycle Length: 145

Actuated Cycle Length: 145

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.82

Intersection Signal Delay: 33.4

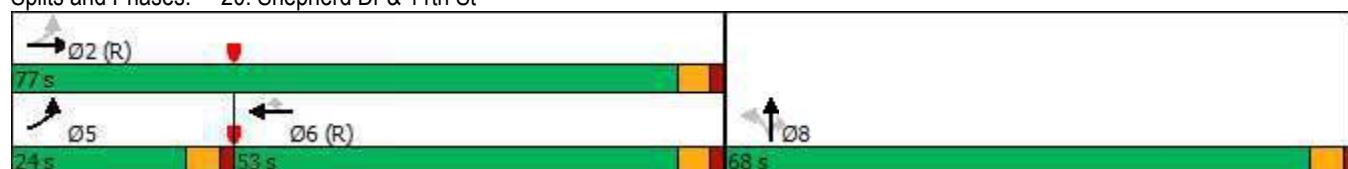
Intersection LOS: C

Intersection Capacity Utilization 87.9%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 20: Shepherd Dr & 11th St



TIRZ 5 TIA
22: Durham Dr & 11th St

2040 Projected Conditions - Scenario 3

Timing Plan: AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑	↑	↑	↑↑					↑	↑↑↑	↑
Traffic Volume (vph)	0	513	228	259	565	0	0	0	0	274	2268	189
Future Volume (vph)	0	513	228	259	565	0	0	0	0	274	2268	189
Satd. Flow (prot)	0	3539	1583	1770	3539	0	0	0	0	1770	5085	1583
Flt Permitted					0.155						0.950	
Satd. Flow (perm)	0	3539	1583	289	3539	0	0	0	0	1770	5085	1583
Satd. Flow (RTOR)				56								114
Lane Group Flow (vph)	0	604	253	328	673	0	0	0	0	285	2577	270
Turn Type	NA	Perm	pm+pt		NA					Perm	NA	Perm
Protected Phases	2			1	6						4	
Permitted Phases			2	6							4	4
Total Split (s)		50.8	50.8	25.2	76.0					69.0	69.0	69.0
Total Lost Time (s)		5.2	5.2	5.2	5.2					5.3	5.3	5.3
Act Effct Green (s)		32.8	32.8	57.9	57.9					76.6	76.6	76.6
Actuated g/C Ratio		0.23	0.23	0.40	0.40					0.53	0.53	0.53
v/c Ratio		0.75	0.63	1.03	0.48					0.31	0.96	0.30
Control Delay		58.3	45.5	80.8	26.5					21.5	43.3	12.6
Queue Delay		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Total Delay		58.3	45.5	80.8	26.5					21.5	43.3	12.6
LOS	E	D	F	C						C	D	B
Approach Delay		54.5		44.3							38.6	
Approach LOS		D		D							D	
Queue Length 50th (ft)	282	170	~287	300						148	830	76
Queue Length 95th (ft)	303	248	#326	312						239	#1031	96
Internal Link Dist (ft)	797			472			1903				4811	
Turn Bay Length (ft)		125	200							200		200
Base Capacity (vph)	1112	536	319	1728						934	2686	890
Starvation Cap Reductn	0	0	0	0						0	0	0
Spillback Cap Reductn	0	0	0	0						0	0	0
Storage Cap Reductn	0	0	0	0						0	0	0
Reduced v/c Ratio	0.54	0.47	1.03	0.39						0.31	0.96	0.30

Intersection Summary

Cycle Length: 145

Actuated Cycle Length: 145

Offset: 69.4 (48%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.03

Intersection Signal Delay: 42.5

Intersection LOS: D

Intersection Capacity Utilization 87.9%

ICU Level of Service E

Analysis Period (min) 15

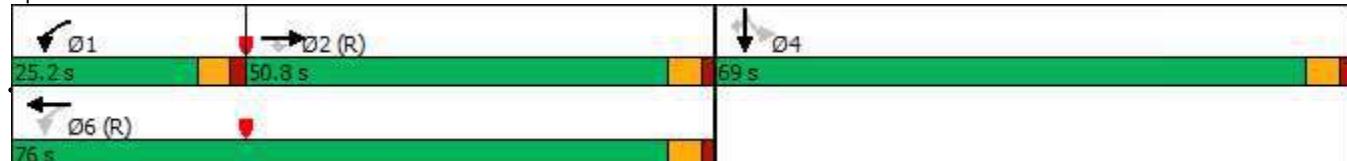
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 22: Durham Dr & 11th St





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑		↑↑	↑↑				
Traffic Volume (vph)	93	393	0	0	272	179	67	2573	134	0	0	0
Future Volume (vph)	93	393	0	0	272	179	67	2573	134	0	0	0
Satd. Flow (prot)	0	3507	0	0	3330	0	0	5035	0	0	0	0
Flt Permitted		0.645						0.998				
Satd. Flow (perm)	0	2283	0	0	3330	0	0	5035	0	0	0	0
Satd. Flow (RTOR)						2		13				
Lane Group Flow (vph)	0	570	0	0	516	0	0	2939	0	0	0	0
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		2				6			8			
Permitted Phases	2							8				
Total Split (s)	45.0	45.0			45.0		75.0	75.0				
Total Lost Time (s)		5.1			5.1			5.3				
Act Effct Green (s)		35.5			35.5			74.1				
Actuated g/C Ratio		0.30			0.30			0.62				
v/c Ratio		0.85			0.52			0.94				
Control Delay		35.5			36.7			29.4				
Queue Delay		0.0			0.0			0.0				
Total Delay		35.5			36.7			29.4				
LOS	D		D			C						
Approach Delay		35.5			36.7			29.4				
Approach LOS	D		D			C						
Queue Length 50th (ft)		248			173			722				
Queue Length 95th (ft)		283			208			#955				
Internal Link Dist (ft)		158			1495			4775			1423	
Turn Bay Length (ft)												
Base Capacity (vph)		759			1108			3115				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.75			0.47			0.94				

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.94

Intersection Signal Delay: 31.2

Intersection LOS: C

Intersection Capacity Utilization 117.0%

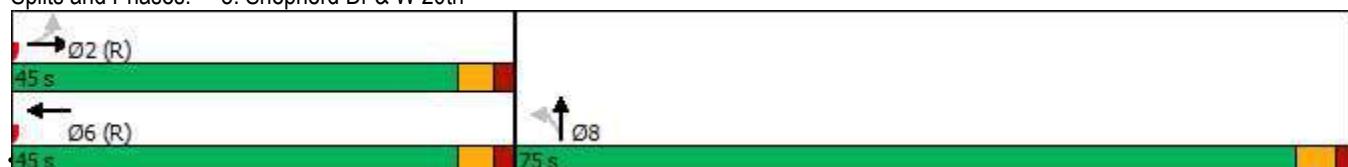
ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 8: Shepherd Dr & W 20th





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↑	↑	↑					↑↑↑	↑↑↑	
Traffic Volume (vph)	0	349	40	104	233	0	0	0	0	138	1628	113
Future Volume (vph)	0	349	40	104	233	0	0	0	0	138	1628	113
Satd. Flow (prot)	0	1863	1583	1770	1863	0	0	0	0	0	5014	0
Flt Permitted					0.203						0.996	
Satd. Flow (perm)	0	1863	1583	378	1863	0	0	0	0	0	5014	0
Satd. Flow (RTOR)				66							13	
Lane Group Flow (vph)	0	447	43	118	265	0	0	0	0	0	1993	0
Turn Type	NA	Perm	pm+pt		NA					Perm	NA	
Protected Phases	2			1	6						4	
Permitted Phases			2	6							4	
Total Split (s)	47.4	47.4	11.6	59.0						61.0	61.0	
Total Lost Time (s)	5.0	5.0	4.5	5.0							5.3	
Act Effct Green (s)	34.1	34.1	44.6	44.1							65.6	
Actuated g/C Ratio	0.28	0.28	0.37	0.37							0.55	
v/c Ratio	0.84	0.09	0.58	0.39							0.73	
Control Delay	55.1	3.0	48.6	39.4							23.3	
Queue Delay	0.0	0.0	0.0	0.0							0.0	
Total Delay	55.1	3.0	48.6	39.4							23.3	
LOS	E	A	D	D							C	
Approach Delay	50.5			42.3							23.3	
Approach LOS	D			D							C	
Queue Length 50th (ft)	325	0	60	144							406	
Queue Length 95th (ft)	337	13	m60	m128							542	
Internal Link Dist (ft)	1438			312			4811				1412	
Turn Bay Length (ft)		10	125									
Base Capacity (vph)	658	602	227	838							2747	
Starvation Cap Reductn	0	0	0	0							0	
Spillback Cap Reductn	0	0	0	0							0	
Storage Cap Reductn	0	0	0	0							0	
Reduced v/c Ratio	0.68	0.07	0.52	0.32							0.73	

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 96.4 (80%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 30.5

Intersection LOS: C

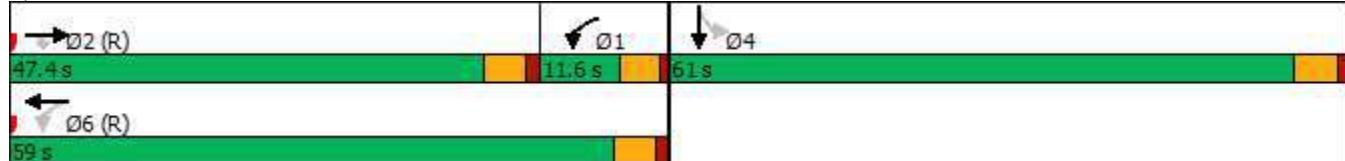
Intersection Capacity Utilization 73.2%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 11: Durham Dr & W 20th St



TIRZ 5 TIA
20: Shepherd Dr & 11th St

2040 Projected Conditions - Scenario 3

Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑			↑↑	↑	↑	↑↑↑	↑			
Traffic Volume (vph)	297	921	0	0	640	155	404	2485	142	0	0	0
Future Volume (vph)	297	921	0	0	640	155	404	2485	142	0	0	0
Satd. Flow (prot)	1770	3539	0	0	3539	1583	1770	5085	1583	0	0	0
Flt Permitted	0.139							0.950				
Satd. Flow (perm)	259	3539	0	0	3539	1583	1770	5085	1583	0	0	0
Satd. Flow (RTOR)						60			62			
Lane Group Flow (vph)	313	1001	0	0	762	199	425	2672	175	0	0	0
Turn Type	pm+pt	NA			NA	Perm	Perm	NA	Perm			
Protected Phases	5	2			6				8			
Permitted Phases	2					6		8		8		
Total Split (s)	21.0	59.0			38.0	38.0	71.0	71.0	71.0			
Total Lost Time (s)	5.2	5.2			5.2	5.2	5.0	5.0	5.0			
Act Effct Green (s)	53.8	53.8			31.9	31.9	66.0	66.0	66.0			
Actuated g/C Ratio	0.41	0.41			0.25	0.25	0.51	0.51	0.51			
v/c Ratio	1.04	0.68			0.88	0.46	0.47	1.04	0.21			
Control Delay	91.9	20.3			59.4	32.2	22.9	59.4	11.8			
Queue Delay	0.0	0.5			0.0	0.0	0.0	0.0	0.0			
Total Delay	91.9	20.8			59.4	32.2	22.9	59.4	11.8			
LOS	F	C			E	C	C	E	B			
Approach Delay	37.8				53.8			52.2				
Approach LOS	D				D			D				
Queue Length 50th (ft)	~229	146			323	98	225	~885	49			
Queue Length 95th (ft)	m#401	194			367	140	316	#972	79			
Internal Link Dist (ft)	472				1073			2200		4775		
Turn Bay Length (ft)	200					100	200		200			
Base Capacity (vph)	300	1464			892	444	898	2581	834			
Starvation Cap Reductn	0	143			0	0	0	0	0			
Spillback Cap Reductn	0	0			0	0	0	0	0			
Storage Cap Reductn	0	0			0	0	0	0	0			
Reduced v/c Ratio	1.04	0.76			0.85	0.45	0.47	1.04	0.21			

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.04

Intersection Signal Delay: 49.0

Intersection LOS: D

Intersection Capacity Utilization 95.0%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

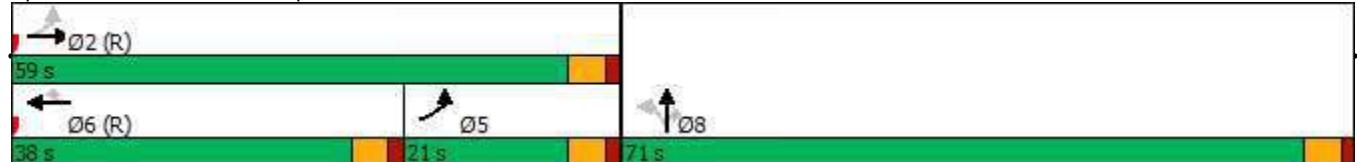
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Shepherd Dr & 11th St





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑	↑	↑	↑↑					↑	↑↑↑	↑
Traffic Volume (vph)	0	1003	219	197	758	0	0	0	0	349	1256	135
Future Volume (vph)	0	1003	219	197	758	0	0	0	0	349	1256	135
Satd. Flow (prot)	0	3539	1583	1770	3539	0	0	0	0	1770	5085	1583
Flt Permitted					0.096						0.950	
Satd. Flow (perm)	0	3539	1583	179	3539	0	0	0	0	1770	5085	1583
Satd. Flow (RTOR)				86								114
Lane Group Flow (vph)	0	1045	255	216	871	0	0	0	0	401	1411	157
Turn Type	NA	Perm	pm+pt	NA						Perm	NA	Perm
Protected Phases	2		1	6							4	
Permitted Phases		2	6							4		4
Total Split (s)	55.5	55.5	25.5	81.0						49.0	49.0	49.0
Total Lost Time (s)	5.2	5.2	5.2	5.2						5.3	5.3	5.3
Act Effct Green (s)	49.4	49.4	71.1	71.1						48.4	48.4	48.4
Actuated g/C Ratio	0.38	0.38	0.55	0.55						0.37	0.37	0.37
v/c Ratio	0.78	0.39	0.72	0.45						0.61	0.75	0.24
Control Delay	40.0	20.4	53.7	7.5						39.1	39.2	10.7
Queue Delay	0.0	0.0	0.0	0.1						0.0	0.0	0.0
Total Delay	40.0	20.4	53.7	7.7						39.1	39.2	10.7
LOS	D	C	D	A						D	D	B
Approach Delay	36.1			16.8							36.9	
Approach LOS	D			B							D	
Queue Length 50th (ft)	390	97	138	113						276	380	24
Queue Length 95th (ft)	485	160	m184	130						388	453	69
Internal Link Dist (ft)	797			472			1903				4811	
Turn Bay Length (ft)		125	200							200		200
Base Capacity (vph)	1394	675	346	2063						658	1893	660
Starvation Cap Reductn	0	0	0	349						0	0	0
Spillback Cap Reductn	0	0	0	0						0	0	0
Storage Cap Reductn	0	0	0	0						0	0	0
Reduced v/c Ratio	0.75	0.38	0.62	0.51						0.61	0.75	0.24

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.78

Intersection Signal Delay: 31.7

Intersection LOS: C

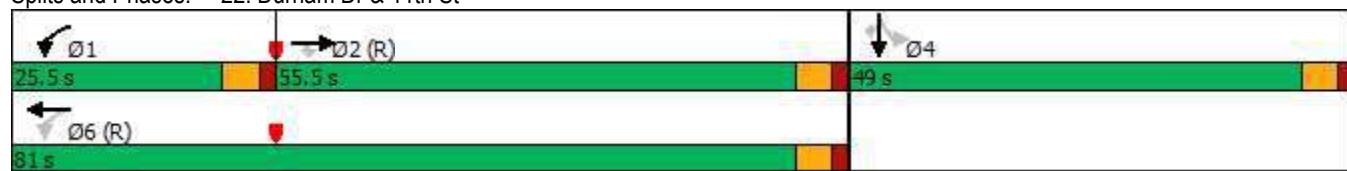
Intersection Capacity Utilization 95.0%

ICU Level of Service F

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 22: Durham Dr & 11th St



Appendix G

Capacity Analysis – 2040 Projected Conditions –

Scenario 4



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑			↑↑↑				
Traffic Volume (vph)	51	231	0	0	261	100	52	1182	84	0	0	0
Future Volume (vph)	51	231	0	0	261	100	52	1182	84	0	0	0
Satd. Flow (prot)	0	3500	0	0	3398	0	0	5019	0	0	0	0
Flt Permitted		0.648						0.998				
Satd. Flow (perm)	0	2293	0	0	3398	0	0	5019	0	0	0	0
Satd. Flow (RTOR)					45			18				
Lane Group Flow (vph)	0	354	0	0	461	0	0	1419	0	0	0	0
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		2				6			8			
Permitted Phases	2							8				
Total Split (s)	45.0	45.0			45.0		75.0	75.0				
Total Lost Time (s)		5.1			5.1			5.3				
Act Effct Green (s)	30.0				30.0			79.6				
Actuated g/C Ratio	0.25				0.25			0.66				
v/c Ratio	0.62				0.52			0.43				
Control Delay	45.4				37.3			9.8				
Queue Delay	0.0				0.0			0.0				
Total Delay	45.4				37.3			9.8				
LOS	D				D			A				
Approach Delay	45.4				37.3			9.8				
Approach LOS	D				D			A				
Queue Length 50th (ft)	128				146			171				
Queue Length 95th (ft)	164				164			199				
Internal Link Dist (ft)	158				1611			4775			1423	
Turn Bay Length (ft)												
Base Capacity (vph)	762				1159			3335				
Starvation Cap Reductn	0				0			0				
Spillback Cap Reductn	0				0			0				
Storage Cap Reductn	0				0			0				
Reduced v/c Ratio	0.46				0.40			0.43				

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.62

Intersection Signal Delay: 21.1

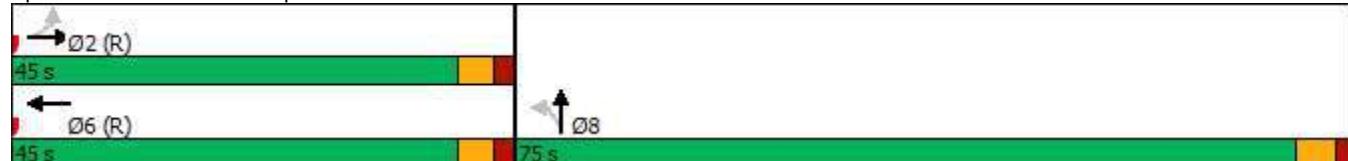
Intersection LOS: C

Intersection Capacity Utilization 88.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 8: Shepherd Dr & W 20th



TIRZ 5 TIA
11: Durham Dr & W 20th St

2040 Projected Conditions - Scenario 4

Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↑	↑	↑					↑↑↑	↑↑↑	
Traffic Volume (vph)	0	155	34	108	214	0	0	0	0	134	2392	163
Future Volume (vph)	0	155	34	108	214	0	0	0	0	134	2392	163
Satd. Flow (prot)	0	1863	1583	1770	1863	0	0	0	0	0	5019	0
Flt Permitted					0.446						0.997	
Satd. Flow (perm)	0	1863	1583	831	1863	0	0	0	0	0	5019	0
Satd. Flow (RTOR)				61							13	
Lane Group Flow (vph)	0	209	42	123	278	0	0	0	0	0	2900	0
Turn Type	NA	Perm	pm+pt	NA						Perm	NA	
Protected Phases	2		1	6							4	
Permitted Phases			2	6							4	
Total Split (s)	45.4	45.4	9.6	55.0						75.0	75.0	
Total Lost Time (s)	5.0	5.0	4.5	5.0							5.3	
Act Effct Green (s)	25.3	25.3	40.5	40.0							79.7	
Actuated g/C Ratio	0.19	0.19	0.31	0.31							0.61	
v/c Ratio	0.58	0.12	0.37	0.49							0.94	
Control Delay	54.5	5.6	40.6	40.1							30.6	
Queue Delay	0.0	0.0	0.0	0.0							0.0	
Total Delay	54.5	5.6	40.6	40.1							30.6	
LOS	D	A	D	D							C	
Approach Delay	46.3			40.3							30.6	
Approach LOS	D			D							C	
Queue Length 50th (ft)	162	0	77	191							771	
Queue Length 95th (ft)	192	12	126	230							857	
Internal Link Dist (ft)	1523			312			4811				1412	
Turn Bay Length (ft)		10	125									
Base Capacity (vph)	578	533	332	716							3082	
Starvation Cap Reductn	0	0	0	0							0	
Spillback Cap Reductn	0	0	0	0							0	
Storage Cap Reductn	0	0	0	0							0	
Reduced v/c Ratio	0.36	0.08	0.37	0.39							0.94	

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 110.8 (85%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.94

Intersection Signal Delay: 32.8

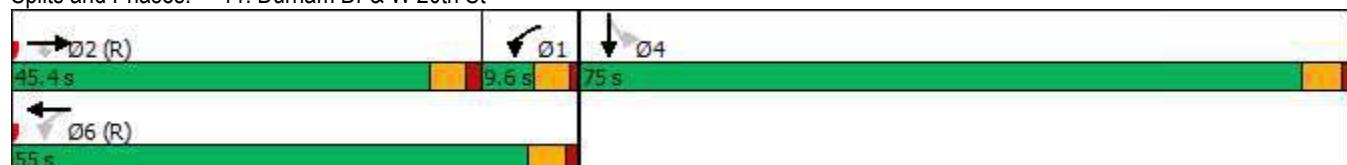
Intersection LOS: C

Intersection Capacity Utilization 94.5%

ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 11: Durham Dr & W 20th St





Lane Group	EBL	EBT	EBC	WBL	WBT	WBC	NBL	NBT	NBC	SBL	SBT	SBC
Lane Configurations	↑↑	↑↑			↑↑		↑↑	↑↑↑	↑↑			
Traffic Volume (vph)	128	649	0	0	703	125	253	1039	153	0	0	0
Future Volume (vph)	128	649	0	0	703	125	253	1039	153	0	0	0
Satd. Flow (prot)	3433	3539	0	0	3451	0	1770	5085	1583	0	0	0
Flt Permitted	0.950						0.950					
Satd. Flow (perm)	3433	3539	0	0	3451	0	1770	5085	1583	0	0	0
Satd. Flow (RTOR)						18			153			
Lane Group Flow (vph)	171	698	0	0	1069	0	278	1129	189	0	0	0
Turn Type	Prot	NA			NA		Perm	NA	Perm			
Protected Phases	5	2			6				8			
Permitted Phases							8		8			
Total Split (s)	24.0	66.0			42.0		64.0	64.0	64.0			
Total Lost Time (s)	5.2	5.2			5.2		5.0	5.0	5.0			
Act Effct Green (s)	12.1	60.8			43.5		59.0	59.0	59.0			
Actuated g/C Ratio	0.09	0.47			0.33		0.45	0.45	0.45			
v/c Ratio	0.54	0.42			0.92		0.35	0.49	0.24			
Control Delay	62.3	23.9			53.9		24.6	25.8	6.0			
Queue Delay	0.0	0.0			0.0		0.0	0.0	0.0			
Total Delay	62.3	23.9			53.9		24.6	25.8	6.0			
LOS	E	C			D		C	C	A			
Approach Delay	31.5				53.9			23.3				
Approach LOS	C				D			C				
Queue Length 50th (ft)	72	201			445		149	239	17			
Queue Length 95th (ft)	88	252			463		219	282	46			
Internal Link Dist (ft)		472			1073			2200			4775	
Turn Bay Length (ft)	200					200		200				
Base Capacity (vph)	496	1655			1166		803	2307	802			
Starvation Cap Reductn	0	0			0		0	0	0			
Spillback Cap Reductn	0	0			0		0	0	0			
Storage Cap Reductn	0	0			0		0	0	0			
Reduced v/c Ratio	0.34	0.42			0.92		0.35	0.49	0.24			

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.92

Intersection Signal Delay: 34.6

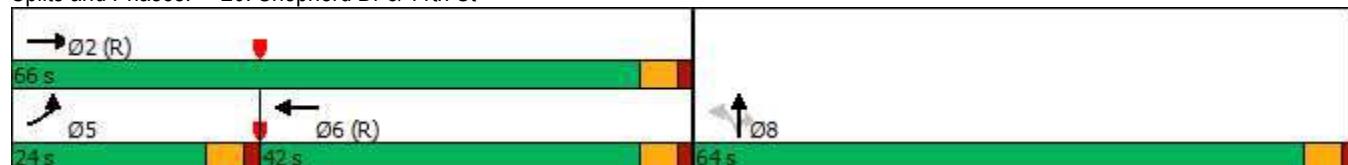
Intersection LOS: C

Intersection Capacity Utilization 87.8%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 20: Shepherd Dr & 11th St



TIRZ 5 TIA
22: Durham Dr & 11th St

2040 Projected Conditions - Scenario 4

Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑	↑	↑↑	↑↑					↑	↑↑↑	
Traffic Volume (vph)	0	513	228	259	565	0	0	0	0	274	2268	189
Future Volume (vph)	0	513	228	259	565	0	0	0	0	274	2268	189
Satd. Flow (prot)	0	3539	1583	3433	3539	0	0	0	0	1770	5014	0
Flt Permitted						0.950					0.950	
Satd. Flow (perm)	0	3539	1583	3433	3539	0	0	0	0	1770	5014	0
Satd. Flow (RTOR)				61							20	
Lane Group Flow (vph)	0	604	253	328	673	0	0	0	0	285	2847	0
Turn Type	NA	Perm	Prot	NA						Perm	NA	
Protected Phases		2		1	6						4	
Permitted Phases			2								4	
Total Split (s)		36.0	36.0	21.0	57.0					78.0	78.0	
Total Lost Time (s)		5.2	5.2	5.2	5.2					5.3	5.3	
Act Effct Green (s)		28.8	28.8	15.4	49.4					75.1	75.1	
Actuated g/C Ratio		0.21	0.21	0.11	0.37					0.56	0.56	
v/c Ratio		0.80	0.66	0.84	0.52					0.29	1.02	
Control Delay		59.1	45.0	77.5	34.9					17.3	51.4	
Queue Delay		0.0	0.0	0.0	0.0					0.0	0.0	
Total Delay		59.1	45.0	77.5	34.9					17.3	51.4	
LOS	E	D	E	C						B	D	
Approach Delay		54.9			48.8						48.3	
Approach LOS		D			D						D	
Queue Length 50th (ft)		262	153	146	235					132	~989	
Queue Length 95th (ft)		308	249	173	268					193	#1031	
Internal Link Dist (ft)		797			472			1903			4811	
Turn Bay Length (ft)			125	200						200		
Base Capacity (vph)		807	408	401	1357					984	2798	
Starvation Cap Reductn	0	0	0	0						0	0	
Spillback Cap Reductn	0	0	0	0						0	0	
Storage Cap Reductn	0	0	0	0						0	0	
Reduced v/c Ratio	0.75	0.62	0.82	0.50						0.29	1.02	

Intersection Summary

Cycle Length: 135

Actuated Cycle Length: 135

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.02

Intersection Signal Delay: 49.6

Intersection LOS: D

Intersection Capacity Utilization 87.8%

ICU Level of Service E

Analysis Period (min) 15

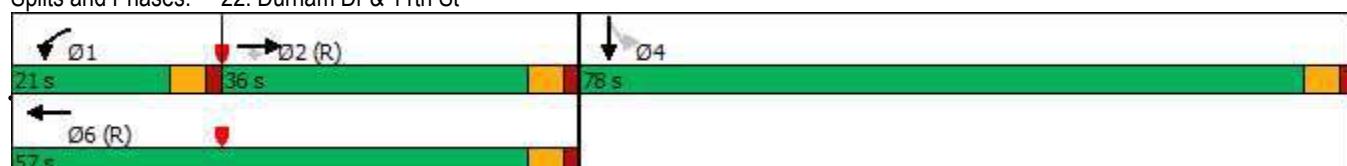
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 22: Durham Dr & 11th St





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑			↑↑		↑↑	↑↑				
Traffic Volume (vph)	93	393	0	0	272	179	67	2573	134	0	0	0
Future Volume (vph)	93	393	0	0	272	179	67	2573	134	0	0	0
Satd. Flow (prot)	0	3507	0	0	3330	0	0	5035	0	0	0	0
Flt Permitted		0.645						0.998				
Satd. Flow (perm)	0	2283	0	0	3330	0	0	5035	0	0	0	0
Satd. Flow (RTOR)						2		13				
Lane Group Flow (vph)	0	570	0	0	516	0	0	2939	0	0	0	0
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		2				6			8			
Permitted Phases	2							8				
Total Split (s)	45.0	45.0			45.0		75.0	75.0				
Total Lost Time (s)		5.1			5.1			5.3				
Act Effct Green (s)		35.5			35.5			74.1				
Actuated g/C Ratio		0.30			0.30			0.62				
v/c Ratio		0.85			0.52			0.94				
Control Delay		35.5			36.7			29.4				
Queue Delay		0.0			0.0			0.0				
Total Delay		35.5			36.7			29.4				
LOS	D		D			C						
Approach Delay		35.5			36.7			29.4				
Approach LOS	D		D			C						
Queue Length 50th (ft)		248			173			722				
Queue Length 95th (ft)		283			208			#955				
Internal Link Dist (ft)		158			1495			4775			1423	
Turn Bay Length (ft)												
Base Capacity (vph)		759			1108			3115				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.75			0.47			0.94				

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.94

Intersection Signal Delay: 31.2

Intersection LOS: C

Intersection Capacity Utilization 117.0%

ICU Level of Service H

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 8: Shepherd Dr & W 20th





Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑	↑	↑	↑					↑↑↑	↑↑↑	
Traffic Volume (vph)	0	349	40	104	233	0	0	0	0	138	1628	113
Future Volume (vph)	0	349	40	104	233	0	0	0	0	138	1628	113
Satd. Flow (prot)	0	1863	1583	1770	1863	0	0	0	0	0	5014	0
Flt Permitted					0.203						0.996	
Satd. Flow (perm)	0	1863	1583	378	1863	0	0	0	0	0	5014	0
Satd. Flow (RTOR)				66							13	
Lane Group Flow (vph)	0	447	43	118	265	0	0	0	0	0	1993	0
Turn Type	NA	Perm	pm+pt		NA					Perm	NA	
Protected Phases	2			1	6						4	
Permitted Phases			2	6							4	
Total Split (s)	47.4	47.4	11.6	59.0						61.0	61.0	
Total Lost Time (s)	5.0	5.0	4.5	5.0							5.3	
Act Effct Green (s)	34.1	34.1	44.6	44.1							65.6	
Actuated g/C Ratio	0.28	0.28	0.37	0.37							0.55	
v/c Ratio	0.84	0.09	0.58	0.39							0.73	
Control Delay	55.1	3.0	48.6	39.4							23.3	
Queue Delay	0.0	0.0	0.0	0.0							0.0	
Total Delay	55.1	3.0	48.6	39.4							23.3	
LOS	E	A	D	D							C	
Approach Delay	50.5			42.3							23.3	
Approach LOS	D			D							C	
Queue Length 50th (ft)	325	0	60	144							406	
Queue Length 95th (ft)	337	13	m60	m128							542	
Internal Link Dist (ft)	1438			312			4811				1412	
Turn Bay Length (ft)		10	125									
Base Capacity (vph)	658	602	227	838							2747	
Starvation Cap Reductn	0	0	0	0							0	
Spillback Cap Reductn	0	0	0	0							0	
Storage Cap Reductn	0	0	0	0							0	
Reduced v/c Ratio	0.68	0.07	0.52	0.32							0.73	

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 96.4 (80%), Referenced to phase 2:EBT and 6:WBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 30.5

Intersection LOS: C

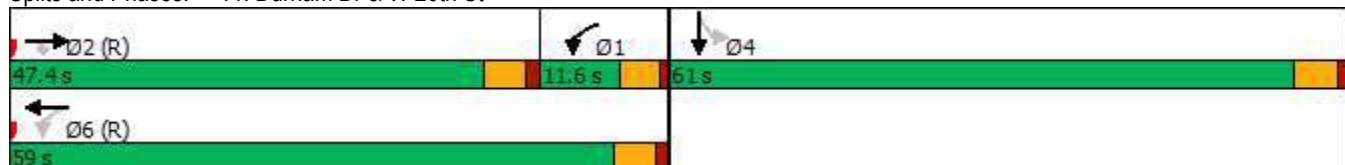
Intersection Capacity Utilization 73.2%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 11: Durham Dr & W 20th St



TIRZ 5 TIA
20: Shepherd Dr & 11th St

2040 Projected Conditions - Scenario 4

Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑			↑↑		↑↑	↑↑↑	↑			
Traffic Volume (vph)	297	921	0	0	640	155	404	2485	142	0	0	0
Future Volume (vph)	297	921	0	0	640	155	404	2485	142	0	0	0
Satd. Flow (prot)	3433	3539	0	0	3429	0	1770	5085	1583	0	0	0
Flt Permitted	0.950						0.950					
Satd. Flow (perm)	3433	3539	0	0	3429	0	1770	5085	1583	0	0	0
Satd. Flow (RTOR)						1			58			
Lane Group Flow (vph)	313	1001	0	0	961	0	425	2672	175	0	0	0
Turn Type	Prot	NA			NA		Perm	NA	Perm			
Protected Phases	5	2			6				8			
Permitted Phases							8		8			
Total Split (s)	15.0	64.0			49.0		76.0	76.0	76.0			
Total Lost Time (s)	5.2	5.2			5.2		5.0	5.0	5.0			
Act Effct Green (s)	10.8	58.8			42.8		71.0	71.0	71.0			
Actuated g/C Ratio	0.08	0.42			0.31		0.51	0.51	0.51			
v/c Ratio	1.18	0.67			0.92		0.47	1.04	0.21			
Control Delay	143.0	18.8			60.8		24.6	62.1	13.2			
Queue Delay	0.0	0.8			0.0		0.0	0.0	0.0			
Total Delay	143.0	19.6			60.8		24.6	62.1	13.2			
LOS	F	B			E		C	E	B			
Approach Delay		49.0			60.8			54.6				
Approach LOS		D			E			D				
Queue Length 50th (ft)	~192	142			440		245	~957	55			
Queue Length 95th (ft)	m#235	m155			481		337	#1040	86			
Internal Link Dist (ft)		472			1073			2200			4775	
Turn Bay Length (ft)	200					200		200				
Base Capacity (vph)	265	1486			1073		897	2578	831			
Starvation Cap Reductn	0	216			0		0	0	0			
Spillback Cap Reductn	0	0			0		0	0	0			
Storage Cap Reductn	0	0			0		0	0	0			
Reduced v/c Ratio	1.18	0.79			0.90		0.47	1.04	0.21			

Intersection Summary

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.18

Intersection Signal Delay: 54.3

Intersection LOS: D

Intersection Capacity Utilization 94.2%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

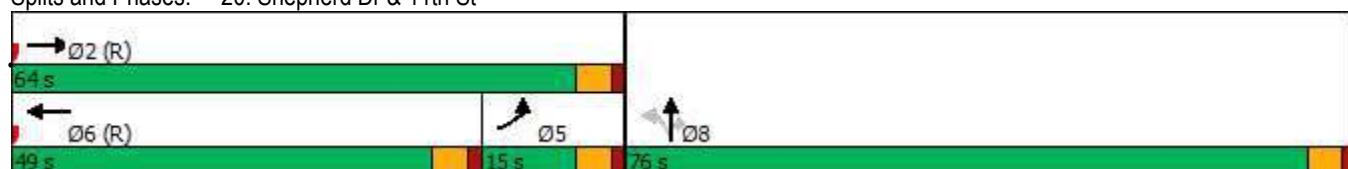
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: Shepherd Dr & 11th St



TIRZ 5 TIA
22: Durham Dr & 11th St

2040 Projected Conditions - Scenario 4

Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑	↗	↖↖	↑↑					↗	↑↑↑	
Traffic Volume (vph)	0	1003	219	197	758	0	0	0	0	349	1256	135
Future Volume (vph)	0	1003	219	197	758	0	0	0	0	349	1256	135
Satd. Flow (prot)	0	3539	1583	3433	3539	0	0	0	0	1770	5009	0
Flt Permitted					0.950						0.950	
Satd. Flow (perm)	0	3539	1583	3433	3539	0	0	0	0	1770	5009	0
Satd. Flow (RTOR)				58							18	
Lane Group Flow (vph)	0	1045	255	216	871	0	0	0	0	401	1568	0
Turn Type	NA	Perm	Prot	NA						Perm	NA	
Protected Phases		2		1	6						4	
Permitted Phases			2								4	
Total Split (s)		49.0	49.0	20.0	69.0					71.0	71.0	
Total Lost Time (s)		5.2	5.2	5.2	5.2					5.3	5.3	
Act Effct Green (s)		44.7	44.7	13.6	63.5					66.0	66.0	
Actuated g/C Ratio		0.32	0.32	0.10	0.45					0.47	0.47	
v/c Ratio		0.93	0.47	0.65	0.54					0.48	0.66	
Control Delay		60.2	32.5	79.3	21.6					27.8	29.8	
Queue Delay		0.0	0.0	0.0	0.4					0.0	0.0	
Total Delay		60.2	32.5	79.3	22.0					27.8	29.8	
LOS	E	C	E	C						C	C	
Approach Delay		54.8			33.4						29.4	
Approach LOS		D			C						C	
Queue Length 50th (ft)		485	143	105	196					246	391	
Queue Length 95th (ft)		#626	216	m128	251					325	437	
Internal Link Dist (ft)		797			472			1903			4811	
Turn Bay Length (ft)			125	200						200		
Base Capacity (vph)		1129	544	362	1612					834	2371	
Starvation Cap Reductn	0	0	0	299						0	0	
Spillback Cap Reductn	0	0	0	0						0	0	
Storage Cap Reductn	0	0	0	0						0	0	
Reduced v/c Ratio	0.93	0.47	0.60	0.66						0.48	0.66	

Intersection Summary

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.93

Intersection Signal Delay: 37.9

Intersection LOS: D

Intersection Capacity Utilization 94.2%

ICU Level of Service F

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 22: Durham Dr & 11th St

